

Revisions: # DATE DESCRIPTION OF CHANGE ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR UTILIZED IN ANY FORM WITHOUT PRIOR WRITTEN AUTHORIZATION OF "COBALT" NOTE: SIGNATURES VALID FOR ONE YEAR ONLY AFTER DATE OF SIGNATURES PROJECT LOCATION OR ADDRESS: CHECKED BY: DRAWN BY: CCH PROJECT #: SCALE: DATE:

RESPONSIBILITIES:

- GENERAL CONTRACTOR IS RESPONSIBLE FOR VERIFYING STRUCTURAL DRAWINGS & ARCHITECTURAL DRAWINGS ARE IN AGREEMENT. ANY DISCOURSE BETWEEN THE TWO SETS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER & ARCHITECT FOR RESOLUTION.
- GENERAL CONTRACTOR IS RESPONSIBLE FOR VERIFYING STRUCTURAL DRAWINGS TO CONFIRM THE AVAILABILITY OF ALL REQUIRED DETAILS. IF ANY REQUIRED INFORMATION IS NOT LISTED IN THE STRUCTURAL DRAWINGS, GENERAL CONTRACTOR IS REQUIRED TO CONTACT ENGINEER OF RECORD FOR PARTICULAR INFORMATION.
- 3. GENERAL CONTRACTOR IS RESPONSIBLE FOR ENSURING CONSTRUCTION ADHERES TO FEMA FLOOD PLAIN, LOCAL MUNICIPALITY, (FFE) FINISH FLOOR ELEVATION REQUIREMENTS AND/OR DISASTER RECOVERY CONSTRUCTION GUIDELINES.
- 4. GENERAL CONTRACTOR IS RESPONSIBLE FOR COMPLIANCE WITH ALL LOCAL MUNICIPALITY CODE REQUIREMENTS AND/OR DISASTER RECOVERY CONSTRUCTION GUIDELINES.
- COBALT ENGINEERING IS NOT RESPONSIBLE FOR DESIGN FLOOD ELEVATION OR ANY REQUIREMENT EXCEEDING THE BASE FLOOD ELEVATION AS SHOWN ON THE PROVIDED ELEVATION CERTIFICATE.
- COBALT ENGINEERING & INSPECTIONS LLC. IS NOT RESPONSIBLE FOR THE PERFORMANCE OF FOUNDATION AS A RESULT OF THE BEHAVIOR OF THE SUPPORTING SOIL AND/OR DIFFERENTIAL SETTLEMENT DUE TO SEASONAL CHANGES SUCH AS DROUGHT, EXTENSIVE RAIN AND OTHER DRASTIC CLIMATE CHANGES

CONCRETE:

- 1. UNLESS OTHERWISE NOTED, ALL CONCRETE FOUNDATION WALLS AND SLABS ON GRADE SHALL BE 3,000 PSI (28 DAY COMPRESSION STRENGTH) CONCRETE, PLACE CONCRETE SLABS ON 4" OF COMPACTED LOW P.I SAND FILL. ALL SLABS UNDER INTERIOR FINISHED AND HEATED LIVING SPACES SHALL BE PLACED ON 6 MIL POLYETHYLENE VAPOR BARRIER WITH A MINIMUM OF 6" LAPPED JOINTS. ALL REBAR LAP SPACING LENGTHS SHALL BE MINIMUM 50 TIMES THE BAR SIZE.
- PROVIDE $\frac{1}{2}$ " EXPANSION JOINT MATERIAL BETWEEN ALL CONCRETE SLABS ABUTTING 2. CONCRETE OR MASONRY WALLS OCCURRING IN EXTERIOR OR UNHEATED SPACES OR AREAS.
- CONCRETE FOR ALL BASEMENT WALLS, FOUNDATION WALLS, PORCHES, WALKS, PATIOS, STEPS, GARAGE, CARPORT FLOOR SLABS AND DRIVEWAYS SHALL BE AIR-ENTRAINED.
- 4. ALL REINFORCEMENT STEEL SHALL MEET ASTM A615 GRADE 60 SPECIFICATIONS.
- REBAR COVER: PROVIDE THE FOLLOWING CONCRETE PROTECTIVE COVERINGS FOR REINFORCEMENT, UNLESS NOTED OTHERWISE.
- 3" FOR ALL CONCRETE DEPOSITED DIRECTLY AGAINST THE GROUND. 2" FOR ALL FORMED CONCRETE EXPOSED TO WEATHER OR IN CONTACT WITH THE Β. GROUND. (UNLESS NOTED OTHERWISE)
- 6. DEVELOPMENTAL LENGTH (DL) SHALL BE 50 TIMES THE DIAMETER OF THE REBAR

RESPONSIBILITIES:

A. ALL (EARTHWORK, EXCAVATIONS, STRUCTURAL FILL, COMPACTION, ETC) SHALL BE DONE IN ACCORDANCE WITH GEO-TECHNICAL REPORT.

1. EXCAVATING

- A. ALL EXCAVATION SHALL COMPLY WITH OSHA STANDARDS
- B. DEMOLISH AND REMOVE ALL OBSTRUCTIONS AS REQUIRED. AREAS TO RECEIVE FOUNDATION SHALL BE STRIPPED TO REMOVE ALL ORGANIC MATERIAL AND CONTAMINATED OR SOFT SOIL. DISPOSAL OF ALL DEBRIS FROM DEMOLITION AND STRIPPING OPERATIONS SHALL BE AS SPECIFIED BY OWNER.
- C. CARE SHALL BE TAKEN TO NOT OVER EXCAVATE BELOW BOTTOM OF FOUNDATION. ANY OVER EXCAVATION REQUIRED FOR REMOVAL OF THE EXISTING FILL SHALL BE BACKFILLED PER SECTION 2.A OR 2.B
- D. ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY PROOF-ROLLING PER TX DOT ITEM 216 TO DETERMINE THAT ALL LOOSE, SOFT, OR OTHERWISE UNDESIRABLE MATERIALS ARE REMOVED. IF AN AREA OF UNDESIRABLE MATERIAL IS DISCOVERED AT THE BOTTOM OF THE EXCAVATION, IT SHALL BE REMOVED AND REPLACED WITH COMPACTED BACKFILL PER SECTION 2.A. OR 2.B.
- E. THE UPPER 6" OF EXPOSED SOILS SHALL BE COMPACTED TO AT LEAST 95% OF MAX. DRY DENSITY DETERMINED BY MODIFIED PROCTOR TEST (ASTM D1557).
- F. WHERE SOIL CONDITIONS PERMIT, FOUNDATIONS BELOW GRADE MAY BE EARTH FORMED UNLESS OTHERWISE NOTED.

2. ENGINEERED FILL (CLAY)

- A. CLAY STRUCTURE FILL SHALL BE SANDY CLAY WITH LIQUID LIMIT OF LESS THAN 35 AND PLASTICITY INDEX (PI) BETWEEN 8 AND 20.
- CLAY SHALL BE MOISTURE CONDITIONED WITHIN 2% OF OPTIMUM MOISTURE CONTENT AND COMPACTED TO AT LEAST 95 % OF THE MAX DRY DENSITY DETERMINED BY THE MODIFIED PROCTOR TEST (ASTM D1557) WITH 8" MAX. LOOSE LIFTS.
- FOR COMPACTION BY MANUALLY-GUIDED POWER COMPACTORS, STRUCTURAL FILL SHALL BE PLACED IN LIFTS OF 6" MAXIMUM LOOSE THICKNESS.

NOTES ON PRESSURE TREATED LUMBER

ALL WOOD MEMBERS (INCLUDING PLYWOOD SHEATHING & ALL WOOD BASED MATERIALS) IN CONTACT WITH CONCRETE, OR EXPOSED TO WEATHER, MOISTURE OR WITHIN 18" OF THE GROUND (SUCH AS PORCH & BALCONY FRAMING) SHALL BE PRESSURE- TREATED.

GALVANIZED

FASTENERS (AND OTHER METAL PRODUCTS) FOR USE WITH WOOD TREATED WITH ACQ PRESERVATIVES INCLUDE: HOT-DIP GALVANIZED (THE MINIMUM STANDARD) THE MINIMUM HOT-DIP GALVANIZED REQUIREMENT FOR USE WITH TREATED WOOD SHOULD CONFORM TO THE FOLLOWING ASTM STANDARDS: ASTM- A153 (FOR HOT-DIP FASTENER PRODUCTS) AND ASTM-A653 (COATING DESIGNATION g-185 FOR HOT-DIP CONNECTOR AND SHEET PRODUCTS).

STAINLESS STEEL STAINLESS STEEL FASTENERS AND CONNECTORS ARE REQUIRED FOR PERMANENT WOOD FOUNDATIONS BELOW GRADE AND ARE RECOMMENDED FOR USE WITH TREATED WOOD IN OTHER SEVERE EXTERIOR APPLICATIONS SUCH AS SWIMMING POOLS, SALT WATER EXPOSURE, ETC. - TYPE 304 AND 316 ARE THE RECOMMENDED GRADES TO US

ALUMINUM SHOULD NOT BE USED IN DIRECT CONTACT WITH PRODUCTS TREATED WITH ACQ PRESERVATIVE SPACER MATERIALS OR OTHER PHYSICAL BARRIERS ARE RECOMMENDED TO PREVENT DIRECT CONTACT OF ACQ TREATED WOOD WITH ALUMINUM PRODUCTS.

STRUCTURAL:

1. IF TRUSSES ARE SPECIFIED ON THE PLANS, THE TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND/OR STRESS AND LOAD CALCULATIONS (DIAGRAMS) FOR CONTRACTORS APPROVAL PRIOR TO CONSTRUCTION. DRAWINGS SHALL BEAR SEAL OF THE REGISTERED ENGINEER IN THE STATE IN WHICH THE STRUCTURE IS BUILT

MISC. BOLTS AND THREADED FASTENERS

- A. SPECIFICATION
- B. DESIGN
- C. INSTALLATION

4. BOLTED CONNECTIONS SHALL BE KNURLED OR SPOT-WELDED TO PREVENT BACK-OUT. 1603.1.4 WIND DESIGN DATA:

THE FOLLOWING INFORMATION RELATED TO WIND LOADS SHALL BE SHOWN, REGARDLESS OF WHETHER WIND LOADS GOVERN THE DESIGN OF THE LATERAL FORCE-RESISTING SYSTEM OF THE STRUCTURE

- RISK CATEGORY.
- UTILIZED.



| Note | s: |
|------------|---|
| 1. | Values are ultimate |
| wind | speeds in miles pe |
| ft (| 10m) above ground f |
| egor | y. |
| 2. | Linear interpolati |
| is p | ermitted. |
| 3. last | Islands and coasta contour shall use our of the coastal |
| 4. | Mountainous terrai |
| prom | ontories, and speci |
| shal | 1 be examined for u |
| tion | S. |
| 5. | Wind speeds corres |
| a 7% | probability of exc |
| (Ann | ual Exceedance Prob |
| MRI | = 700 years). |

CURRENTLY, THE PRODUCT COMMONLY USED FOR PRESSURE TREATMENT IS ALKALINE COPPER QUATERNARY (ACQ). THIS MATERIAL IS EXTREMELY CORROSIVE. ONLY HOT- DIPPED

ANCHOR BOLTS, THRU BOLTS, NAILS, OR OTHER CORROSIVE-RESISTANT FASTENERS, SHALL BE USED WITH ACQ-TREATED LUMBER. FASTENER MANUFACTURER OR SUPPLIER SHALL BE CONSULTED ON THE SUITABILITY OF GALVANIZED FASTENERS FOR USE WITH TREATED LUMBER.

1. BOLTS SHALL CONFIRM TO ASTM A307 GRADE A. CARBON STEEL EXTERNALLY THREADEDFASTENERS, U.N.O.

2. BOLTS AND NUTS SHALL BE HEX HEAD ASTM A307 AND CONFORM TO ANSI STANDARDS B18.2.1 AND B18.2.1 AS WELL AS ASTM MATERIAL STANDARDS ASTM 307. 3. WASHERS SHALL BE CIRCULAR, FLAT AND SMOOTH IN CONFORMANCE WITH THE

REQUIREMENTS OF TYPE A WASHERS IN ANSI STANDARD B23.1.

1. THE MINIMUM BOLT DIAMETER SHALL BE ¹/₂ INCH AND BE BEARING TYPE CONNECTION USING STANDARD HOLES WITH THREADS EXCLUDED FROM THE SHEAR PLANE, U.N.O.

1. A307 BOLTS SHALL BE TIGHTENED PER TURN-OF-NUT BOLTING METHOD. THE TURN-OF-NUT BOLTING METHOD CONDITION DEFINED AS THE TIGHTNESS ATTAINED BY THE EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH. THEN BACKED OFF 2/3 TURN. (PER AISC MANUEL OF STEEL CONSTRUCTION, LOAD & RESISTANCE FACTOR 3RD EDITION 8.2.1) DO NOT OVERTIGHTEN BOLTS AS TO DAMAGE THE WOOD ELEMENTS.

2. BOLTS AND NUTS SHALL BE WELL LUBRICATED AT TIME OF INSTALLATION. DRY. CORRODED BOLTS WILL NOT BE ALLOWED.

ALL BOLTS SHALL BE NEW AND NOT REUSED.

BASIC DESIGN WIND SPEED, V, MILES PER HOUR AND ALLOWABLE STRESS DESIGN WIND SPEED, VASD, AS DETERMINED IN ACCORDANCE WITH SECTION 1609.3.1.

WIND EXPOSURE. APPLICABLE WIND DIRECTION IF MORE THAN ONE WIND EXPOSURE IS

APPLICABLE INTERNAL PRESSURE COEFFICIENT

DESIGN WIND PRESSURES TO BE USED FOR EXTERIOR COMPONENT AND CLADDING MATERIALS NOT SPECIFICALLY DESIGNED BY THE REGISTERED DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN OF THE STRUCTURE, PSF (KN/M2).

ZONE LEGEND

ZONE 5 AS INDICATED ARE ZONE 4

THE ALPHA VALUE DEFINES THE ANY OUTSIDE CORNER)

FLORIDA WIND BORNE DEBRIS PROTECTION **TABLE OF CONTENTS** XX-XXXX-GN-1.00 GENERAL NOTES ' THIS STRUCTURE IS IN A WIND BORNE DEBRIS PROTECTION AREAS. IT IS WITHIN 1 MILE OF THE XX-XXXX-GN-2.00 **GENERAL NOTES 2** COASTAL MEAN HIGH WATER LINE WHERE THE ULTIMATE WIND SPEED, Vult. AND IS 130MPH OR XX-XXXX-S-1.00 FOUNDATION PLAN GREATER WIND ZONE. XX-XXXX-S-2.00 STRINGER PLAN OR XX-XXXX-S-3.00 FLOOR JOIST PLAN IN AN AREA WHERE THE ULTIMATE DESIGN WIND SPEED, Vult. IS 140MPH OR GREATER. XX-XXXX-S-4.00 CEILING JOIST PLAN WIND BORNE DEBRIS PROTECTIONS SHALL MATCH OR EXCEED THE DESIGN PRESSURE FOR THE XX-XXXX-S-5.00 RAFTER PLAN **OPENING BEING COVERED** XX-XXXX-S-6.00 SHEARWALL & HOLD-DOWN PLAN XX-XXXX-SD-1.00 STANDARD DETAILS 1 **CODES & DESIGN LOADS** XX-XXXX-SD-2.00 **STANDARD DETAILS 2** XX-XXXX-SD-3.00 **STANDARD DETAILS 3** CODE: XX-XXXX-SD-4.00 STANDARD DETAILS 4 FLORIDA BUILDING CODE- 2020 (CITY OF FORT PIERCE REQUIREMENTS) XX-XXXX-SD-5.00 **STANDARD DETAILS 5** LEGEND (MIN.) DESIGN PRESSURE FOR WINDSTORM COMPLIANCE) ALL AREAS NOT DESIGNATED AS TYPICAL DETAIL OR SECTION CALLOUT DETAIL PAGE WINDOWS, DOORS AND WALLS SIZE OF ZONE 5 (MEASURED FROM (XX/XX-X.XX) **ABBREVIATIONS** $\alpha = 3'-0"$ SHOULD BE 10% OF SHORTEST WALL ANTHONY POWER BEAM APB. VERIFY WITH PLANS APP. ANTHONY POWER PRESERVED DESIGN PRESSURE (PSF) CANT CANTILEVER CONT. CONTINUOUS LL COVERING GARAGE DOOR ROOF DWG. DRAWING +19/-30 --------FND. FOUNDATION +19/-53 --------F.V. FIELD VERIFY +19/-79 ----____ HDR. HEADER PLCS PLACES ____ ____ ----SIM. SIMILAR ----____ ____ S.F. STEEL FLITCH +25/-27 +28/-34 ----SOUTHERN YELLOW PINE SYP. EXPOSURE = CTRIPLE TRPL INTERNAL PRESSURE COEFFICIENT = 0.18 TYP. TYPICAL NOTES 1. ROOF LIVE LOADS 20 PSF (SUBJECT TO SLOPE & TRIBUTARY AREA REDUCTION FACTORS) 2. FLOOR LIVE LOADS 1. ALL SPECIFICATIONS ARE MINIMUM (PSF) USE LOAD (PSF **REQUIREMENTS. SPECIFICATIONS MAY BE** SLEEPING ROOMS 30 INCREASED TO MEET ARCHITECTURAL OR CONSTRUCTION PREFERENCES. **OTHER ROOMS** 40 ATTIC W/STORAGE 20 (b) THIS PLAN IS INTENDED TO BE PLOTTED AT (c) ATTIC W/O STORAGE 10 (b) 24"X36". IF PLOTTED AT ANY OTHER SIZE IT GUARDRAILS & HANDRAILS 250(d) GARAGE N/A(a) WILL NOT BE TO SCALE. THIS INCLUDES, BU (a) ELEVATED GARAGE FLOORS SHALL BE CAPABLE OF SUPPORTING A NOT LIMITED TO DIMENSIONS & ENGINEERS 2,000-POUND LOAD APPLIED OVER A 20-SQUARE-INCH AREA. SEAL. (b) NO STORAGE LOAD IS REQUIRED WITH ROOF SLOPES OF 3 IN 12, OR FLATTER. Revisions: (c) INDIVIDUAL STAIR TREADS SHALL BE DESIGNED FOR THE DATE DESCRIPTION OF CHANGE # UNIFORMLY DISTRIBUTED LIVE LOAD OR A 300-POUNDS CONCENTRATED LOAD ACTING OVER AN AREA OF 4 SQUARE INCHES, WHICHEVER PRODUCES THE GREATER STRESSES (d) A SINGLE CONCENTRATED LOAD APPLIED IN ANY DIRECTION @ ANY POINT ALONG THE TOP. 3. WIND LOADS (AMERICAN SOCIETY OF CIVIL ENGINEERS, 7-16) ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR UTILIZED IN ANY FORM WITHOUT PRIOR WRITTEN AUTHORIZATION OF "COBALT" BASIC WIND DESIGN VELOCITY 160 MPH (ULTIMATE WIND SPEED) RISK CATEGORY: II 4. DEAD LOAD IM WALL FRAMING REQUIREMENTS ANCHOR BOLT REQUIREMENTS MATERIAL & SPACING 5/8"X10" J-BOLTS W/ 3" SQ. X NOTE: SIGNATURES VALID FOR ONE YEAR ONLY AFTER DATE OF SIGNATURES 2X6 SYP #2 @ 16" O.C. .125"THK. WASHER @ 32" O.C. 2X4 SYP #2 @ 16" O.C. 2X6 SYP #2 @ 16" O.C. NOTES: 1. WALL SHEATHING SHALL BE AS INDICATED ON SHEARWALL & HOLDOWN PLAN. EXTERIOR COVERINGS SHALL CONFORM TO FBC INSTALLATION INSTRUCTIONS. DIMENSIONS: 1. DRAWING DIMENSIONS GOVERN OVER SCALE. VERIFY ALL ROUGH OPENING DIMENSIONS FOR SELECTED DOORS, WINDOWS AND MECHANICAL REQUIREMENTS BEFORE CONSTRUCTION BEGINS. FASTENER CORROSION RESISTANCE MOISTURE EXPOSURE FINISH EXTERIOR HOT DIPPED GALV. (MIN.) ELECTRO GALVANIZED (MIN.) ENCLOSED BUT VENTILATED EPOXY COATED (MIN.) AIR CONDITIONED SPACE AW CHECKED BY DRAWN BY: CCH N.T.S. SCALE: PROJECT #:

| | COMPONENT L | | |
|------|-------------|---------|-----|
| ZONE | DOOR | WINDOW | WAI |
| 1 | | | |
| 2 | | | |
| 3 | | | |
| 4 | +32/-34 | +34/-36 | |
| 5 | +32/-41 | +34/-45 | |
| ALL | | | |

PRESSURE AREA 10 SQ. FT.

DESIGN LOADS:

| USE | LOAD (|
|--------------------|--------|
| EXTERIOR BALCONIES | 40 |
| DECKS | 40 |
| FIRE ESCAPES | 40 |
| STAIRS/RAMPS | 40 |
| | |

| USE | LOAD (PSF |
|-------|-----------|
| ROOF | 10 |
| DECKS | 10 |
| FLOOR | 10 |
| WALLS | 11 |

| | MINIMU |
|------------------|----------------|
| | WALL TYPE |
| 1 STORY HOUSE | EXTERIOR WALLS |
| | INTERIOR WALLS |
| | PLUMBING WALLS |
| | |

DATE:

GN-1.00

DRAWING LIST

| NAILING | SCHEDULE | | |
|--|---------------------------|------------------------|---------------------|
| JOINT DESCRIPTION | NUMBER OF COMMON NAILS | NUMBER OF BOX NAILS | NAIL SPACING |
| WALL FRA | MING | | |
| Top plate to top plate (face-nailed) | 2-16d | 2-16d | per foot |
| Top plates at intersections (face-nailed) | 4-16d | 5-16d | joints - each side |
| Stud to stud (face-nailed) | 2-16d | 2-16d | 24"o.c. |
| Header to header (face-nailed) | 16d | 16d | 16"o.c. along edges |
| Top or bottom plate to stud (end-nailed) | 1 -16d | 1 -40d | per stud |
| Bottom plate to floor joist, bandjoist, endjoist or blocking (face-nai | iled) 2-16d | 2-16d | per foot |
| FLOOR FR/ | AMING | | |
| Joist to sill, top plate or girder (toe-nailed) | 4-8d | 4-10d | per joist |
| Bridging to joist (toe-nailed) | 2-8d | 2-10d | each end |
| Blocking to joist (toe-nailed) | 2-8d | 2-10d | each end |
| Blocking to sill or top plate (toe-nailed) | 3-16d | 4-16d | each block |
| Ledger strip to beam (face-nailed) | 3-16d | 4-16d | each joist |
| Joist on ledger to beam (toe-nailed) | 3-8d | 3-10d | per joist |
| Band joist to joist (end-nailed) | 3-16d | 4-16d | per joist |
| Band joist to sill or top plate (toe-nailed) | 2-16d | 3-16d | per foot |
| CEILING SHE | EATHING | | |
| Gypsum wallboard | 5d coolers | 5d coolers | 7"edge/10"field |
| ROOF FRA | MING | | |
| Rafter to top plate (toe-nailed) | 5 -8d | 5 -10d | per rafter |
| Ceiling joist to top plate (toe-nailed) | 5 -8d | 5 -10d | per joist |
| Ceiling joist to parallel rafter (face-nailed) | 7 -16d | 7 -40d | each lap |
| Ceiling joist laps over partitions (face-nailed) | 7 -16d | 7 -40d | each lap |
| Collar tie to rafter (face-nailed) | 3 -8d | 3 -10d | per tie |
| Blocking to rafter (toe-nailed) | 2-8d | 2 -10d | each end |
| Rim board to rafter (end-nailed) | 2-16d | 3 -16d | each end |

| CONCRETE COVERAGE FOR CAST-IN-PLACE (NON PRE-STRESSED CONCRETE MEMBERS) | | | | | |
|--|---|--|--------------------|--|--|
| CONCRETE EXPOSURE | MEMBER | REINFORCEMENT | SPECIFIED COVER | | |
| CAST AGAINST AND PERMANENTLY IN CONTACT W/ GROUND | ALL | ALL | 3" | | |
| EXPOSED TO WEATHER OR IN CONTACT WITH GROUND | ALL | NO. 6 THROUGH NO. 18 BARS | 2" | | |
| | | NO. 5, W31 / D31 WIRE & SMALLER | 1-1⁄2" | | |
| NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND | SLABS, | NO. 14 & NO. 18 BARS | 1-1⁄2" | | |
| | WALLS | NO. 11 BARS & SMALLER | ³ ⁄4" | | |
| | BEAMS, COLUMNS, PEDESTALS, & TENSION | PRIMARY REINFORCEMENT, STIRRUPS, TIES, SPIRALS, & HOOPS | 1-1⁄2" | | |

| DEVELOPMENT LENGTH CHART | | | | | | |
|--------------------------|--------------------------|-------------------------------|-----|--------|-------|--|
| SLAB/ MATS WALLS | | | | | LLS | |
| BAR SIZE | 12" THICKNESS OR LESS | THICKNESS GREATER THAN 12" | | HORIZ. | VERT. | |
| | ALL BARS | BOTTOM OTHER BARS BARS | | | | |
| #3 | 17" | 17" | 22" | 17" | 22" | |
| #4 | 22" | 22" | 30" | 22" | 30" | |
| #5 | 29" | 29" | 36" | 29" | 36" | |

-ONSTRUCTION



| R | evisions: | | | |
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| # | DATE | DE | SCRIPTION OF CHANG | iΕ |
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| ALL F UTILIZ | L RIGHTS RESERV ZED IN ANY FOF | L ED. NO PART OF THI M WITHOUT PRIOR | S DOCUMENT MAY BE RE WRITTEN AUTHORIZATION | PRODUCED OR N OF "COBALT". |
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| DRAW | N BY: | AW | CHECKED BY: | ССН |
| PROJE | ECT #: | | SCALE: | N.T.S. |
| DATE: | | | | GN-2.00 |



FOUNDATION PLAN (STANDARD) 01

ALL OPTIONS

N.T.S.

| | 2X12X12 SILL PLATE W/ TERMITE BARRIER BELOW |
|-----|--|
| | 12X12X8 FILLED CMU BLOCKS |
| | (4) #5 REBAR W/ 12" BEND & #3 TIES 12" O.C. VERTICALLY |
| EQ. | (4) #4 REBAR MATT (T&B) W/ #3 TIES 9" O.C.E.W. |
| | |

0.148"ØX3" 10D NAILS EACH JOIST TO STRINGER W/ LOLLIPOP

JOIST TO JOIST W/ (6)

FLOOR DECKING PER PLAN

STRINGERS TO CMU PIER W/ SIMPSON HDU4-SDS2.5 HOLD-DOWN (ANCHOR BOLT MAY BE CAST IN PLACE W/ CAPATURED WASHER OR DRILLED & EPOXIED W/ SIMPSON SET-XP EPOXY IN $\frac{3}{4}$ "Ø HOLE *

JOIST PER PLAN

BOTTOM PLATE

2X6 EXTERIOR WALL

| DRAWN BY: | AW | CHECKED BY: | ССН |
|------------|----|-------------|--------------|
| PROJECT #: | | SCALE: | 1/4" = 1'-0" |
| DATE: | | | S-1.00 |

| L | EG | ΕN | ID |
|---|----|----|----|
| | | | |

| x | DETAIL PAGE # | | | | |
|--|--|--|--|--|--|
| TYPE X Pxx | TYPE OF NOTCH SIZE OF PILING (P06= 6x6) FOR PILING DEPTH DETAIL #6 DWG. SD-1.00 (SEE UNLESS NOTED OTHERWISE ON PLAN) | | | | |
| Cxx | SIZE OF CMU BLOCK (C12= 12X12) (SEE SECTIONS A & B THIS DWG. FOR PIER CONSTRUCTION SECTIONS) UNLESS NOTED OTHERWISE ON PLAN) | | | | |
| | | | | | |
| | | | | | |
| | NOTES | | | | |
| CONO 1. SE | CRETE HOLD-DOWNS: EE DRAWING S-6.00 FOR TYPE OF | | | | |
| 2. SE IN M/ IN | EE DRAWING S-1.00 FOR HOLD-DOWN STALLATION DETAIL. (REFER TO ANUFACTURERS INSTALLATION STRUCTIONS FOR ADDITIONAL REQ'S.) | | | | |
| Revis | DATE DESCRIPTION OF CHANGE | | | | |
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| ALL RIGHT UTILIZED IN | L TS RESERVED. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR N ANY FORM WITHOUT PRIOR WRITTEN AUTHORIZATION OF "COBALT". | | | | |
| THIS DRAWING IS A CONCEPTUAL PLAN AND IS NOT INTENDED TO BE USED AS A SITE SPECIFIC PLAN NOR SHALL IT BE USED FOR PERMITTING PURPOSES. SITE SPECIFICS SHALL BE PREPARED ON A CASE-BY-CASE BASIS AND ANY USE BEYOND THE AFOREMENTIONED INTENT IS PROHIBITED. | | | | | |
| NOTE: SIG | NATURES VALID FOR ONE YEAR ONLY AFTER DATE OF SIGNATURES | | | | |
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| | PROJECT LOCATION OR ADDRESS: | | | | |

02 STRINGER PLAN (STANDARD) ALL OPTIONS



LEGEND

DESIGNATES 6X6 HOLD DOWN BLOCK.

| R | evisions: | | | | | | |
|-----------------|---------------------------------|---------------------------------------|--|--|--|--|--|
| # | DATE | DE | SCRIPTION OF CHANGE | | | | |
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| ALL F UTILIZ | RIGHTS RESERV ZED IN ANY FOF | ED. NO PART OF THI M WITHOUT PRIOR | S DOCUMENT MAY BE REPRODUCED OR WRITTEN AUTHORIZATION OF "COBALT". | | | | |
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| IN | TENDED TO | D BE USED AS A | A SITE SPECIFIC PLAN NOR | | | | |
| SH SF | HALL IT BE PECIFICS S | USED FOR PEF HALL BE PREP | RMITTING PURPOSES. SITE ARED ON A CASE-BY-CASE | | | | |
| BA | ASIS AND A | NY USE BEYON | ND THE AFOREMENTIONED | | | | |
| | | INTENTIOF | | | | | |
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03 FLOOR JOIST PLAN (STANDARD) ALL OPTIONS



| | | | | | LEG | END | |
|-----------|--------------------------------------|--|---------------------------|--|---|--|---|
| (5) | 2X10 | LUS210 OR LUC210Z CONCEALED | | | | | |
| 6 | (2) 2X10 | HUS210-2 OR HUC210-2 CONCEALED | | | | | |
| H RECC | OT DIPPED GAL OMMENDED ON EXPO | VANIZED OR STAINLESS STEEL EXTERIOR HANGERS WITH DIRECT SURE TO MOISTURE | | | | | |
| SC | ME MODELS MA | AY NOT BE USED (SEE PLAN FOR LOCATION) | _ | | | | |
| | | | | | | | |
| | | | | | | | |
| | | | | | NO | TES | |
| | | | FLC 1. | OOR FRAM HOUSE 16" O.C | IING: FLOOR JOIS . (U.N.O.) | TS SHALL BE 2X | (12 #2 SYP @ |
| | | | 2. | DECK F 16" O.C | LOOR JOIST | S SHALL BE 2X1 | 0 #2 SYP @ |
| | | | 3. | DOUBL ABOVE | E THE FLOOF RUN PARALI | R JOIST WHERE LEL TO FLOOR J | WALLS OIST. |
| | | | 4. | FLOOR 0.120 x DIAPHF PATTEF | DECKING: ³ / ₄ 3" NAILS, 6" <i>4</i> RAGM. 6" EDG RN. (SEE DET | " APA RATED PL AROUND PERIME SE & 12" FIELD N AIL #3 DWG. SD | YWOOD ETER OF AIL -2.00) |
| | | | FRA 1. | AMING: ALL ST ETC. SI DIRECT | UDS, FLOOR HALL VERTIC, TLY ON EACH | JOISTS, RAFTEF ALLY ALIGN AND OTHER. | RS, TRUSS,) BEAR |
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LEGEND

BRACE POINT

BW - BRACE TO WALL BJ - BRACE TO JOIST BB - BRACE TO BEAM BS - BRACE TO STRONGBACK

NOTES

ROOF FRAMING: 1. RIDGES TO BE 2x10 #2 SYP

- 2. ALL HIPS TO BE 2x8 #2 SYP
- 3. ALL RAFTERS TO BE 2x6 #2 SYP 16"O.C. (UNO)
- ROOF SHEATHING: ¹⁹/₃₂" APA RATED PLYWOOD W/ 0.120 X 3" RING SHANK NAILS, 4" AROUND PERIMETER OF DIAPHRAGM. 6" EDGE & 6" FIELD NAIL PATTERN. PER SECTION R803
- 5. ROOF UNDERLAYMENT SHALL COMPLY WITH R905.1.1.1 & TABLE R905.1.1.1

FRAMING:

1. ALL STUDS, FLOOR JOISTS, RAFTERS, TRUSS, ETC. SHALL VERTICALLY ALIGN AND BEAR DIRECTLY ON EACH OTHER.

| Revisions: | | | | | |
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NOTE: SIGNATURES VALID FOR ONE YEAR ONLY AFTER DATE OF SIGNATURES

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| SHEAR WALLS: ALL SHEARWALLS SHALL EXTEND TO ROOF DIAPHRAGM OR NEXT FLOOR DIAPHRAGM. Image: Strategy of the | SHEAR WALLS ALL SHEARWALLS SHALL EXTEND TO ROOF DIAPHRAGM OR NEXT FLOOR DIAPHRAGM. Image: State of the state of th | | LEGEND |
|---|---|--|---|
| SW11 19/32" APA RATED PLYWOOD SHEATHING 6" PERIMETER AND W/6" EDGE / 12" FIELD NAIL PATTERN USING 0.120 X 3" NAILS SW4 SW41 19/32" APA RATED PLYWOOD SHEATHING 4" PERIMETER AND W/4" EDGE / 12" FIELD NAIL PATTERN USING 0.120 X 3" NAILS HOLD-DOWNS: ALL HOLD-DOWNS SHALL BE SECURED TO MIN. DBL STUD OR GREATER PER MANUFACTURERS INSTRUCTIONS. Image: Manufacture per manu | SW11 19/32" APA RATED PLYWOOD SHEATHING 6" PERIMETER AND W(#" EDGE / 12" FIELD NAIL PATTERN USING 0.120 x 3" NAILS SW4 SW4: 19/32" APA RATED PLYWOOD SHEATHING 4" PERIMETER AND W4" EDGE / 12" FIELD NAIL PATTERN USING 0.120 x 3" NAILS HOLD-DOWNS: ALL HOLD-DOWNS: ALL HOLD-DOWNS SHALL BE SECURED TO MIN. DBL STUD OR GREATER PER MANUFACTURERS INSTRUCTIONS. M1 MSTC68B3 W/ (16) 10D NAILS IN BEAM & (38) 10D NAILS IN STUD PACK PER MANUFACTURERS INSTRUCTIONS M2 MSTC68B3 W/ (18) 10D NAILS IN BEAM & (38) 10D NAILS IN STUD PACK PER MANUFACTURERS INSTRUCTIONS M2 MSTC68B3 W/ (18) 10D NAILS IN BEAM & (38) 10D NAILS IN STUD PACK PER MANUFACTURERS INSTRUCTIONS M2 MSTC68B3 W/ (18) 10D NAILS IN BEAM & (38) 10D NAILS IN STUD PACK PER MANUFACTURERS INSTRUCTIONS M2 MSTC68B3 W/ (18) 10D NAILS IN BEAM & (38) 10D NAILS IN STRUCTIONS M3 DATE MANUFACTURERS INSTRUCTIONS PER MANUFACTURERS INSTRUCTIONS (SEE HOU TO FLOOR BEAM CONNECTION DETAIL ON THIS DWG. FOR INSTALLATION TO FLOOR BEAM). * DRAG STRUT BEAM W/ SIMP. HGA10KT AT EACH END (SEE DETAIL S-4.00) * DRAG STRUT BEAM W/ SIMP. HGA10KT AT EACH END (SEE DETAIL S-4.00) * DRAG STRUT PERORED ON CHANGE MULLED IN ANY FORM WITHOUT PRIOR WRITEN AUTHORIZATION OF "COBALT". ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR UTILEED IN ANY FORM WITHOUT PRIOR WRITEN AUTHORIZATION OF "COBALT". | <u>SHEAR V</u> ALL SHE DIAPHR# | WALLS: ARWALLS SHALL EXTEND TO ROOF AGM OR NEXT FLOOR DIAPHRAGM. |
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TYPICAL EMBEDDED PORCH POST DETAIL

| KING AND JACK STUD SCHEDULE | | | | | |
|-----------------------------|---|--|--|--|--|
| OPENING SPAN | KING/JACK STUDS REQUIRED PER OPENING SPAN | | | | |
| 4' OR LESS | (1) KING & (1) JACK STUD (EACH SIDE) | | | | |
| 4' TO 6' OPENING | (2) KING & (2) JACK STUD (EACH SIDE) | | | | |
| 6' TO 8' OPENING | (3) KING & (2) JACK STUD (EACH SIDE) | | | | |
| 8' TO 12' OPENING | (4) KING & (3) JACK STUD (EACH SIDE) | | | | |
| 12' TO 16' OPENING | (5) KING & (4) JACK STUD (EACH SIDE) | | | | |

| BEND | MSTA3 |
|------|---------|
| BACK | SIDE O |
| PACK | (6" DO\ |
| STUD | MIN.) |
| | , |

| | SINGLE STORY HEADER SCHEDULE | | | | | |
|-------|-----------------------------------|-----------------|---|--|--|--|
| ALL H | ALL HEADERS ARE DROP BEAMS, U.N.O | | | | | |
| TYPE | OPENING SPAN | WALL THICKNE | HEADER REQ'D SS | | | |
| 1 | 3' OR LESS | 3.5" | TWO 2"x8" #2 SYP w/ 1/2" PLYWD FLITCH | | | |
| 2 | 3' OR LESS | 5.5" | THREE 2"x8" #2 SYP w/ 1/2" PLYWD FLITCHS | | | |
| 3 | 3' TO 5' | 3.5" | TWO 2"x10" #2SYP w/ 1/2" PLYWD FLITCH | | | |
| 4 | 3' TO 5' | 5.5" | THREE 2"x10" #2SYP w/ 1/2" PLYWD FLITCHS | | | |
| 5 | 5' TO 8' | 3.5" | TWO 2"x12" #2 SYP w/ 1/2" PLYWD FLITCH | | | |
| 6 | 5' TO 8' | 5.5" | THREE 2"x12" #2 SYP w/ 1/2" PLYWD FLITCHS | | | |
| | | | | | | |

ALL NON-LOAD BEARING HEADERS SHALL BE 2X6 #2 SYP W/ $\frac{1}{2}$ " PLYWOOD FLITCH.

| 2x6 RAIL CAP | | | | | |
|--|---------------------|---|---|---|---|
| 2x4 TOP & BOTTOM RAILS ATTACHED TO GUARD POST W/(2)8d THREADED NAILS OR (2)#8 SCREWS 2 ¹ / ₂ " LONG ON INSIDE FACE | | | | | |
| 2x2 BALUSTERS @ 5" O.C. MAX. ATTACH TO TOP AND BOTTOM RAILS W/(1)#8 WOOD SCREW OR (2)8d (0.135"0) THREADED NAILS | | | | | |
| MIN. 4x4 POST @ 6'-0"O.C. MA ATTACHED TO DECK JOIST W/(2 ¹ / ₂ " GALV. THRU-BOLTS. DO NOT NOTCH POST | | | | | |
| BLOCKING ON EACH SIDE OF POST | | | | | |
| | | | | | |
| (3) ¹ / ₂ " GALV. THRU-BOLTS (H.D.G.) BAND JOIST PER PLAN | | | | | |
| | | | | | |
| EN BLOCKING TOGETHER WITH (12) NAILS (0.131"x2-1/2") | | | | | |
| DECK RAIL INSTALLATION DETAIL | | | | | |
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| 10d | 8d | 10d |
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| | | | | | | | | | | | | | | | | |

| | S |) | 1 | 10 | 1 | 1 | 1 | 2 | | 13 | | 14 | | 15 | | 16 |
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| | ç | Э | | 10 | | 11 | 1 | 12 | | 13 | , | 14 | 1 | 5 | | 16 |
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| 0d | 8d | 10d |
| 24 | 42 | 28 | 46 | 32 | 52 | 35 | 56 | 38 | 61 | 41 | 66 | 44 | 70 | 47 | 75 | 50 |

FEMA BREAKAWAY WALL FASTENING TABLE

NOTES

- THIS DRAWING MAY NOT APPLY IF THE 1. STRUCTURE IS NOT IN A FLOOD ZONE.
- FEMA APPROVED FLOOD VENTS ARE 2. REQUIRED WHEN ENCLOSED SPACE IS BELOW B.F.E. IN AN "A" FLOOD ZONE.
- FEMA APPROVED BREAKAWAY WALLS OR 3. LOUVERS ARE REQUIRED WHEN ENCLOSED SPACE IS BELOW B.F.E. IN A "V" FLOOD ZONE. (EXCEPTION: UP TO 299 SQ. FT. MAY BE ENCLOSED W/O BREAKAWAY OR LOUVERED WALLS.)
- ALL MATERIALS BELOW B.F.E. SHALL BE 4. FLOOD DAMAGE RESISTANT MATERIAL, INCLUDING, BUT NOT LIMITED TO TREATED LUMBER. SEE FEMA TECHNICAL BULLETIN #2 FOR ADDITIONAL INFORMATION.

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| U | ALL F | RIGHTS RESERV ED IN ANY FOR | ED. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR RAW WITHOUT PRIOR WRITTEN AUTHORIZATION OF "COBALT". |

THIS DRAWING IS A CONCEPTUAL PLAN AND IS NOT INTENDED TO BE USED AS A SITE SPECIFIC PLAN NOR SHALL IT BE USED FOR PERMITTING PURPOSES. SITE SPECIFICS SHALL BE PREPARED ON A CASE-BY-CASE BASIS AND ANY USE BEYOND THE AFOREMENTIONED INTENT IS PROHIBITED.

| DRAWN BY: | | CCH |
|------------|--------|---------|
| PROJECT #: | SCALE: | N.T.S. |
| DATE: | | SD-5.00 |

DESIGN AND CONSTRUCTION:

ALL BREAKAWAY WALLS DESIGNED USING THE PRESCRIPTIVE DESIGN METHOD SHALL BE DETAILED IN ACCORDANCE WITH THE FOLLOWING:

1. BREAKAWAY WALLS SHALL BE DESIGNED TO MEET ALL APPLICABLE LOCAL REQUIREMENTS AND BUILDING CODE REQUIREMENTS.

2. AS SHOWN IN BREAKAWAY WALL SECTION (THIS DWG.), WOOD-FRAMED AND STEEL STUD-FRAMED BREAKAWAY WALL PANELS SHALL NOT BE ATTACHED TO THE PILINGS OR OTHER VERTICAL FOUNDATION MEMBERS. ONLY THE TOPS AND BOTTOMS OF WALL PANELS SHALL BE CONNECTED TO PERMANENT 2X4 NAILER PLATES. HIGH-CAPAC ITY CONNECTORS SUCH AS BOLTS, LAG SCREWS, METAL STRAPS, OR HURRICANE FASTENERS (E.G., CLIPS OR STRAPS) SHALL

3. THE EXTERIOR SHEATHING ON BREAKAWAY WALL PANELS SHALL NEITHER OVERLAP NOR BE ATTACHED TO THE VERTICAL FOUNDATION MEMBERS.

4. BREAKAWAY WALL SHEATHING AND SIDING SHALL BE DISCONTINUOUS AT ELEVATED FLOOR BEAMS AND JOISTS; HORIZONTAL SEPARATION JOINTS SHALL BE PROVIDED TO PREVENT DAMAGE TO THE SHEATHING OR SIDING ABOVE THE FLOOR OF THE ELEVATED BUILDING.

5. UTILITIES, INCLUDING ELECTRICAL WIRING, BREAKER BOXES, POWER METERS, PLUMBING, CONDUITS, AND VENTILATION DUCTS, SHALL NOT BE PLACED IN OR ATTACHED TO BREAKAWAY WALL PANELS.

6. BREAKAWAY WALL PANELS SHALL BE POSITIONED SUCH THAT, ON FAILURE, THEY DO NOT COLLAPSE AGAINST CROSS-BRACING OR THREATEN OTHER FOUNDATION COMPONENTS.*FOR MORE INFORMATION, SEE TECHNICAL

7. PARTIAL HEIGHT BREAKAWAY WALL SYSTEMS ARE NOT

RESPONSIBILITIES:

- 1. GENERAL CONTRACTOR IS RESPONSIBLE FOR VERIFYING STRUCTURAL DRAWINGS & ARCHITECTURAL DRAWINGS ARE IN AGREEMENT. ANY DISCOURSE BETWEEN THE TWO SETS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER & ARCHITECT FOR RESOLUTION.
- 2. GENERAL CONTRACTOR IS RESPONSIBLE FOR VERIFYING STRUCTURAL DRAWINGS TO CONFIRM THE AVAILABILITY OF ALL REQUIRED DETAILS. IF ANY REQUIRED INFORMATION IS NOT LISTED IN THE STRUCTURAL DRAWINGS, GENERAL CONTRACTOR IS REQUIRED TO CONTACT ENGINEER OF RECORD FOR PARTICULAR INFORMATION.
- GENERAL CONTRACTOR IS RESPONSIBLE FOR ENSURING CONSTRUCTION ADHERES TO FEMA FLOOD PLAIN, LOCAL MUNICIPALITY, (FFE) FINISH FLOOR ELEVATION REQUIREMENTS AND/OR DISASTER RECOVERY CONSTRUCTION GUIDELINES
- GENERAL CONTRACTOR IS RESPONSIBLE FOR COMPLIANCE WITH ALL LOCAL MUNICIPALITY CODE REQUIREMENTS AND/OR DISASTER RECOVERY CONSTRUCTION GUIDELINES.
- 5. COBALT ENGINEERING IS NOT RESPONSIBLE FOR DESIGN FLOOD ELEVATION OR ANY REQUIREMENT EXCEEDING THE BASE FLOOD ELEVATION AS SHOWN ON THE PROVIDED ELEVATION CERTIFICATE.
- COBALT ENGINEERING & INSPECTIONS LLC. IS NOT RESPONSIBLE FOR THE PERFORMANCE OF FOUNDATION AS A RESULT OF THE BEHAVIOR OF THE SUPPORTING SOIL AND/OR DIFFERENTIAL SETTLEMENT DUE TO SEASONAL CHANGES SUCH AS DROUGHT. EXTENSIVE RAIN AND OTHER DRASTIC CLIMATE CHANGES

CONCRETE:

- 1. UNLESS OTHERWISE NOTED, ALL CONCRETE FOUNDATION WALLS AND SLABS ON GRADE SHALL BE 3,000 PSI (28 DAY COMPRESSION STRENGTH) CONCRETE, PLACE CONCRETE SLABS ON 4" OF COMPACTED LOW P.I SAND FILL. ALL SLABS UNDER INTERIOR FINISHED AND HEATED LIVING SPACES SHALL BE PLACED ON 6 MIL POLYETHYLENE VAPOR BARRIER WITH A MINIMUM OF 6" LAPPED JOINTS. ALL REBAR LAP SPACING LENGTHS SHALL BE MINIMUM 50 TIMES THE BAR SIZE.
- 2. PROVIDE ¹/₂" EXPANSION JOINT MATERIAL BETWEEN ALL CONCRETE SLABS ABUTTING CONCRETE OR MASONRY WALLS OCCURRING IN EXTERIOR OR UNHEATED SPACES OR AREAS.
- 3. CONCRETE FOR ALL BASEMENT WALLS, FOUNDATION WALLS, PORCHES, WALKS, PATIOS, STEPS, GARAGE, CARPORT FLOOR SLABS AND DRIVEWAYS SHALL BE AIR-ENTRAINED.
- 4. ALL REINFORCEMENT STEEL SHALL MEET ASTM A615 GRADE 60 SPECIFICATIONS
- 5. REBAR COVER: PROVIDE THE FOLLOWING CONCRETE PROTECTIVE COVERINGS FOR REINFORCEMENT, UNLESS NOTED OTHERWISE.
- A. 3" FOR ALL CONCRETE DEPOSITED DIRECTLY AGAINST THE GROUND B. 2" FOR ALL FORMED CONCRETE EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND. (UNLESS NOTED OTHERWISE)
- 6. DEVELOPMENTAL LENGTH (DL) SHALL BE 50 TIMES THE DIAMETER OF THE REBAR

RESPONSIBILITIES:

A. ALL (EARTHWORK, EXCAVATIONS, STRUCTURAL FILL, COMPACTION, ETC) SHALL BE DONE IN ACCORDANCE WITH GEO-TECHNICAL REPORT.

1. EXCAVATING

- A. ALL EXCAVATION SHALL COMPLY WITH OSHA STANDARDS
- DEMOLISH AND REMOVE ALL OBSTRUCTIONS AS REQUIRED. AREAS TO RECEIVE FOUNDATION SHALL BE STRIPPED TO REMOVE ALL ORGANIC MATERIAL AND CONTAMINATED OR SOFT SOIL. DISPOSAL OF ALL DEBRIS FROM DEMOLITION AND STRIPPING OPERATIONS SHALL BE AS SPECIFIED BY OWNER.
- C. CARE SHALL BE TAKEN TO NOT OVER EXCAVATE BELOW BOTTOM OF FOUNDATION. ANY OVER EXCAVATION REQUIRED FOR REMOVAL OF THE EXISTING FILL SHALL BE BACKFILLED PER SECTION 2.A OR 2.B
- D. ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY PROOF-ROLLING PER TX DOT ITEM 216 TO DETERMINE THAT ALL LOOSE, SOFT, OR OTHERWISE UNDESIRABLE MATERIALS ARE REMOVED. IF AN AREA OF UNDESIRABLE MATERIAL IS DISCOVERED AT THE BOTTOM OF THE EXCAVATION, IT SHALL BE REMOVED AND REPLACED WITH COMPACTED BACKFILL PER SECTION 2.A. OR 2.B.
- E. THE UPPER 6" OF EXPOSED SOILS SHALL BE COMPACTED TO AT LEAST 95% OF MAX. DRY DENSITY DETERMINED BY MODIFIED PROCTOR TEST (ASTM D1557).
- F. WHERE SOIL CONDITIONS PERMIT, FOUNDATIONS BELOW GRADE MAY BE EARTH FORMED UNLESS OTHERWISE NOTED.

2. ENGINEERED FILL (CLAY)

- CLAY STRUCTURE FILL SHALL BE SANDY CLAY WITH LIQUID LIMIT OF LESS THAN 35 AND PLASTICITY INDEX (PI) BETWEEN 8 AND 20.
- B. CLAY SHALL BE MOISTURE CONDITIONED WITHIN 2% OF OPTIMUM MOISTURE CONTENT AND COMPACTED TO AT LEAST 95 % OF THE MAX DRY DENSITY DETERMINED BY THE MODIFIED PROCTOR TEST (ASTM D1557) WITH 8" MAX. LOOSE LIFTS.
- C. FOR COMPACTION BY MANUALLY-GUIDED POWER COMPACTORS. STRUCTURAL FILL SHALL BE PLACED IN LIFTS OF 6" MAXIMUM LOOSE THICKNESS.

NOTES ON PRESSURE TREATED LUMBER:

CURRENTLY, THE PRODUCT COMMONLY USED FOR PRESSURE TREATMENT IS ALKALINE COPPER QUATERNARY (ACQ). THIS MATERIAL IS EXTREMELY CORROSIVE. ONLY HOT- DIPPED GALVANIZED ANCHOR BOLTS, THRU BOLTS, NAILS, OR OTHER CORROSIVE-RESISTANT FASTENERS, SHALL BE USED WITH ACQ-TREATED LUMBER. FASTENER MANUFACTURER OR SUPPLIER SHALL BE CONSULTED ON THE SUITABILITY OF GALVANIZED FASTENERS FOR USE WITH TREATED LUMBER.

FASTENERS (AND OTHER METAL PRODUCTS) FOR USE WITH WOOD TREATED WITH ACQ PRESERVATIVES INCLUDE: HOT-DIP GALVANIZED (THE MINIMUM STANDARD) THE MINIMUM HOT-DIP GALVANIZED REQUIREMENT FOR USE WITH TREATED WOOD SHOULD CONFORM TO THE FOLLOWING ASTM STANDARDS: ASTM- A153 (FOR HOT-DIP FASTENER PRODUCTS) AND ASTM-A653 (COATING DESIGNATION g-185 FOR HOT-DIP CONNECTOR AND SHEET PRODUCTS).

STAINLESS STEEL STAINLESS STEEL FASTENERS AND CONNECTORS ARE REQUIRED FOR PERMANENT WOOD FOUNDATIONS BELOW GRADE AND ARE RECOMMENDED FOR USE WITH TREATED WOOD IN OTHER SEVERE EXTERIOR APPLICATIONS SUCH AS SWIMMING POOLS, SALT WATER EXPOSURE, ETC. - TYPE 304 AND 316 ARE THE RECOMMENDED GRADES TO US

ALUMINUM SHOULD NOT BE USED IN DIRECT CONTACT WITH PRODUCTS TREATED WITH ACQ PRESERVATIVE SPACER MATERIALS OR OTHER PHYSICAL BARRIERS ARE RECOMMENDED TO PREVENT DIRECT CONTACT OF ACQ TREATED WOOD WITH ALUMINUM PRODUCTS.

STRUCTURAL:

IF TRUSSES ARE SPECIFIED ON THE PLANS, THE TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND/OR STRESS AND LOAD CALCULATIONS (DIAGRAMS) FOR CONTRACTORS APPROVAL PRIOR TO CONSTRUCTION. DRAWINGS SHALL BEAR SEAL OF THE REGISTERED ENGINEER IN THE STATE IN WHICH THE STRUCTURE IS BUILT.

MISC. BOLTS AND THREADED FASTENERS

- A. SPECIFICATION

- B. DESIGN
- C. INSTALLATION

1603.1.4 WIND DESIGN DATA:

- THE STRUCTURE:
- RISK CATEGORY.
- UTILIZED.

| 150 | 3 140 | A. |
|-----|-------|----|
| | | |

FIG. 1609.3(1), 2020 FBC

ALL WOOD MEMBERS (INCLUDING PLYWOOD SHEATHING & ALL WOOD BASED MATERIALS) IN CONTACT WITH CONCRETE. OR EXPOSED TO WEATHER. MOISTURE OR WITHIN 18" OF THE GROUND (SUCH AS PORCH & BALCONY FRAMING) SHALL BE PRESSURE- TREATED.

1. BOLTS SHALL CONFIRM TO ASTM A307 GRADE A. CARBON STEEL EXTERNALLY THREADEDFASTENERS, U.N.O.

2. BOLTS AND NUTS SHALL BE HEX HEAD ASTM A307 AND CONFORM TO ANSI STANDARDS B18.2.1 AND B18.2.1 AS WELL AS ASTM MATERIAL STANDARDS ASTM 307.

3. WASHERS SHALL BE CIRCULAR, FLAT AND SMOOTH IN CONFORMANCE WITH THE **REQUIREMENTS OF TYPE A WASHERS IN ANSI STANDARD B23.1.**

1. THE MINIMUM BOLT DIAMETER SHALL BE $\frac{1}{2}$ INCH AND BE BEARING TYPE CONNECTION USING STANDARD HOLES WITH THREADS EXCLUDED FROM THE SHEAR PLANE, U.N.O.

1. A307 BOLTS SHALL BE TIGHTENED PER TURN-OF-NUT BOLTING METHOD. THE TURN-OF-NUT BOLTING METHOD CONDITION DEFINED AS THE TIGHTNESS ATTAINED BY THE EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH, THEN BACKED OFF 2/3 TURN. (PER AISC MANUEL OF STEEL CONSTRUCTION, LOAD & RESISTANCE FACTOR 3RD EDITION 8.2.1) DO NOT OVERTIGHTEN BOLTS AS TO DAMAGE THE WOOD ELEMENTS.

BOLTS AND NUTS SHALL BE WELL LUBRICATED AT TIME OF INSTALLATION, DRY, CORRODED BOLTS WILL NOT BE ALLOWED.

ALL BOLTS SHALL BE NEW AND NOT REUSED.

BOLTED CONNECTIONS SHALL BE KNURLED OR SPOT-WELDED TO PREVENT BACK-OUT.

THE FOLLOWING INFORMATION RELATED TO WIND LOADS SHALL BE SHOWN, REGARDLESS OF WHETHER WIND LOADS GOVERN THE DESIGN OF THE LATERAL FORCE-RESISTING SYSTEM OF

1. BASIC DESIGN WIND SPEED, V, MILES PER HOUR AND ALLOWABLE STRESS DESIGN WIND SPEED, VASD, AS DETERMINED IN ACCORDANCE WITH SECTION 1609.3.1.

WIND EXPOSURE. APPLICABLE WIND DIRECTION IF MORE THAN ONE WIND EXPOSURE IS

APPLICABLE INTERNAL PRESSURE COEFFICIENT

DESIGN WIND PRESSURES TO BE USED FOR EXTERIOR COMPONENT AND CLADDING MATERIALS NOT SPECIFICALLY DESIGNED BY THE REGISTERED DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN OF THE STRUCTURE, PSF (KN/M2).

DRAWING LIST FLORIDA WIND BORNE DEBRIS PROTECTION TABLE OF CONTENTS XX-XXXX-GN-1.00 GENERAL NOTES 1 XX-XXXX-GN-2.00 **GENERAL NOTES 2** THIS STRUCTURE IS IN A WIND BORNE DEBRIS PROTECTION AREAS. IT IS WITHIN 1 MILE OF THE COASTAL MEAN HIGH WATER LINE WHERE THE ULTIMATE WIND SPEED. Vult. AND IS 130MPH OR XX-XXXX-F-1.00 FOUNDATION & CMU WALL PLANS XX-XXXX-F-2.00 FOUNDATION DETAILS XX-XXXX-S-1.00 **CEILING JOIST & RAFTER PLANS** IN AN AREA WHERE THE ULTIMATE DESIGN WIND SPEED, Vult. IS 140MPH OR GREATER. XX-XXXX-SD-1.00 **STANDARD DETAILS 1** XX-XXXX-SD-2.00 STANDARD DETAILS 2 WIND BORNE DEBRIS PROTECTIONS SHALL MATCH OR EXCEED THE DESIGN PRESSURE FOR THE FLORIDA BUILDING CODE- 2020 (CITY OF FORT PIERCE REQUIREMENTS) (MIN.) DESIGN PRESSURE FOR WINDSTORM COMPLIANCE) TYPICAL DETAIL OR SECTION CALLOUT PAGE DETAIL RTEST WALL (XX/XX-X,XX)ESIGN PRESSURE (PSF) ABBREVIATIONS _ COVERING GARAGE DOOR ROOF APB. ANTHONY POWER BEAM +19/-30 ----APP. ANTHONY POWER PRESERVED +19/-53 CANT CANTILEVER --------CONT. CONTINUOUS +19/-79 --------DF. DOUGLAS FIR ------------DWG. DRAWING --------FND. FOUNDATION +25/-27 +28/-34 ----F.V. FIELD VERIFY HDR. HEADER PLACES PLCS. SIM. SIMILAR S.F. STEEL FLITCH SPF. SPRUCE PINE FIR SYP. SOUTHERN YELLOW PINE 1. ROOF LIVE LOADS 20 PSF (SUBJECT TO SLOPE & TRIBUTARY AREA REDUCTION FACTORS) TRPL TRIPLE TYP. TYPICAL PSF) USE LOAD (PSF NOTES SLEEPING ROOMS 30 OTHER ROOMS 40 ATTIC W/STORAGE 20 (b) ALL SPECIFICATIONS ARE MINIMUM ATTIC W/O STORAGE **REQUIREMENTS. SPECIFICATIONS MAY BE** 10 (b) INCREASED TO MEET ARCHITECTURAL OR i0(d) GARAGE N/A(a) CONSTRUCTION PREFERENCES. (a) ELEVATED GARAGE FLOORS SHALL BE CAPABLE OF SUPPORTING A 2,000-POUND LOAD APPLIED OVER A 20-SQUARE-INCH AREA (b) NO STORAGE LOAD IS REQUIRED WITH ROOF SLOPES OF 3 IN 12, OR Revisions: DATE DESCRIPTION OF CHANGE # (c) INDIVIDUAL STAIR TREADS SHALL BE DESIGNED FOR THE UNIFORMLY DISTRIBUTED LIVE LOAD OR A 300-POUNDS CONCENTRATED LOAD ACTING OVER AN AREA OF 4 SQUARE INCHES, WHICHEVER PRODUCES THE GREATER STRESSES. (d) A SINGLE CONCENTRATED LOAD APPLIED IN ANY DIRECTION @ ANY ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR 3. WIND LOADS (AMERICAN SOCIETY OF CIVIL ENGINEERS, 7-16) UTILIZED IN ANY FORM WITHOUT PRIOR WRITTEN AUTHORIZATION OF "COBALT" BASIC WIND DESIGN VELOCITY 160 MPH (ULTIMATE WIND SPEED) **RISK CATEGORY: II** THIS DRAWING IS A CONCEPTUAL PLAN AND IS NOT INTENDED TO BE USED AS A SITE SPECIFIC PLAN NOR SHALL IT BE USED FOR PERMITTING PURPOSES. SITE SPECIFICS SHALL BE PREPARED ON A CASE-BY-CASE BASIS AND ANY USE BEYOND THE AFOREMENTIONED INTENT IS PROHIBITED. MINIMUM WALL FRAMING REQUIREMENTS MATERIAL & SPACING ANCHOR BOLT REQUIREMENTS NOTE: SIGNATURES VALID FOR ONE YEAR ONLY AFTER DATE OF SIGNATURES 5/8"X10" J-BOLTS W/ 3" SQ. X 2X6 SYP #2 @ 16" O.C. .125"THK. WASHER @ 32" O.C. 2X4 SYP #2 @ 16" O.C. 2X6 SYP #2 @ 16" O.C. 1. WALL SHEATHING SHALL BE AS INDICATED ON SHEARWALL & HOLDOWN PLAN. EXTERIOR COVERINGS SHALL CONFORM TO FBC INSTALLATION INSTRUCTIONS. 1. DRAWING DIMENSIONS GOVERN OVER SCALE. VERIFY ALL ROUGH OPENING DIMENSIONS FOR SELECTED DOORS, WINDOWS AND MECHANICAL REQUIREMENTS BEFORE CONSTRUCTION RESISTANCE FINISH OT DIPPED GALV. (MIN.) CTRO GALVANIZED (MIN. POXY COATED (MIN.) DRAWN BY: AW CHECKED BY CCH N.T.S. SCALE: PROJECT #:

GREATER WIND ZONE. OR

OPENING BEING COVERED.

CODES & DESIGN LOADS

CODE:

ZONE LEGEND

ALL AREAS NOT DESIGNATED AS ZONE 5 AS INDICATED ARE ZONE 4

WINDOWS, DOORS AND WALLS THE ALPHA VALUE DEFINES THE SIZE OF ZONE 5 (MEASURED FROM ANY OUTSIDE CORNER)

| COMPONENT | DE |
|-----------|----|

| ZONE | DOOR | WINDOW | WALI |
|------|---------|---------|------|
| 1 | | | |
| 2 | | | |
| 3 | | | |
| 4 | +32/-34 | +34/-36 | |
| 5 | +32/-41 | +34/-45 | |
| ALL | | | |

EXPOSURE = C

INTERNAL PRESSURE COEFFICIENT = 0.18 PRESSURE AREA 10 SQ. FT.

DESIGN LOADS:

2. FLOOR LIVE LOADS

| USE | LOAD (F |
|-----------------------------------|---------|
| EXTERIOR BALCONIES | 40 |
| DECKS | 40 |
| FIRE ESCAPES | 40 |
| STAIRS/RAMPS | 40 (|
| GUARDRAILS & HANDRAILS | 250 |
| | |

- FLATTER.
- POINT ALONG THE TOP.

4. DEAD LOAD

| USE | LOAD (PSF) |
|-------|------------|
| ROOF | 10 |
| DECKS | 10 |
| FLOOR | 10 |
| WALLS | 11 |

WALL TYPE

| | EXTERIOR WALLS |
|-------|----------------|
| HOUSE | INTERIOR WALLS |
| | PLUMBING WALLS |
| | |

NOTES

DIMENSIONS:

BEGINS.

| FASTENER CORRC | SION |
|-------------------------|------|
| MOISTURE EXPOSURE | |
| EXTERIOR | HO |
| ENCLOSED BUT VENTILATED | ELEC |
| AIR CONDITIONED SPACE | E |

| NAILING | SCHEDULE | | |
|--|---------------------------|------------------------|---------------------|
| JOINT DESCRIPTION | NUMBER OF COMMON NAILS | NUMBER OF BOX NAILS | NAIL SPACING |
| WALL FRA | MING | | |
| Top plate to top plate (face-nailed) | 2-16d | 2-16d | per foot |
| Top plates at intersections (face-nailed) | 4-16d | 5-16d | joints - each side |
| Stud to stud (face-nailed) | 2-16d | 2-16d | 24"o.c. |
| Header to header (face-nailed) | 16d | 16d | 16"o.c. along edges |
| Top or bottom plate to stud (end-nailed) | 1 -16d | 1 -40d | per stud |
| Bottom plate to floor joist, bandjoist, endjoist or blocking (face-nai | iled) 2-16d | 2-16d | per foot |
| FLOOR FR/ | AMING | | |
| Joist to sill, top plate or girder (toe-nailed) | 4-8d | 4-10d | per joist |
| Bridging to joist (toe-nailed) | 2-8d | 2-10d | each end |
| Blocking to joist (toe-nailed) | 2-8d | 2-10d | each end |
| Blocking to sill or top plate (toe-nailed) | 3-16d | 4-16d | each block |
| Ledger strip to beam (face-nailed) | 3-16d | 4-16d | each joist |
| Joist on ledger to beam (toe-nailed) | 3-8d | 3-10d | per joist |
| Band joist to joist (end-nailed) | 3-16d | 4-16d | per joist |
| Band joist to sill or top plate (toe-nailed) | 2-16d | 3-16d | per foot |
| CEILING SHE | EATHING | | |
| Gypsum wallboard | 5d coolers | 5d coolers | 7"edge/10"field |
| ROOF FRA | MING | | |
| Rafter to top plate (toe-nailed) | 5 -8d | 5 -10d | per rafter |
| Ceiling joist to top plate (toe-nailed) | 5 -8d | 5 -10d | per joist |
| Ceiling joist to parallel rafter (face-nailed) | 7 -16d | 7 -40d | each lap |
| Ceiling joist laps over partitions (face-nailed) | 7 -16d | 7 -40d | each lap |
| Collar tie to rafter (face-nailed) | 3 -8d | 3 -10d | per tie |
| Blocking to rafter (toe-nailed) | 2-8d | 2 -10d | each end |
| Rim board to rafter (end-nailed) | 2-16d | 3 -16d | each end |

| CONCRETE (NON PRE-S | COVERAGE I TRESSED CO | FOR CAST-IN-PLACE |) |
|---|---|--|--------------------|
| CONCRETE EXPOSURE | MEMBER | REINFORCEMENT | SPECIFIED COVER |
| CAST AGAINST AND PERMANENTLY IN CONTACT W/ GROUND | ALL | ALL | 3" |
| EXPOSED TO WEATHER | | NO. 6 THROUGH NO. 18 BARS | 2" |
| WITH GROUND | ALL | NO. 5, W31 / D31 WIRE & SMALLER | 1-1⁄2" |
| | SLABS, | NO. 14 & NO. 18 BARS | 1-1⁄2" |
| | WALLS | NO. 11 BARS & SMALLER | ³ ⁄4" |
| CONTACT WITH GROUND | BEAMS, COLUMNS, PEDESTALS, & TENSION | PRIMARY REINFORCEMENT, STIRRUPS, TIES, SPIRALS, & HOOPS | 1-1⁄2" |

| | DEVELOPMENT LENGTH CHART | | | | | | | |
|-------------|--------------------------|-------------------------------|---------------------------|--------|-------|--|--|--|
| | SLAB/ | MATS | | WA | LLS | | | |
| BAR SIZE | 12" THICKNESS OR LESS | THICKNESS GREATER THAN 12" | | HORIZ. | VERT. | | | |
| | ALL BARS | BOTTOM BARS | BOTTOM OTHER BARS BARS | | | | | |
| #3 | 17" | 17" | 22" | 17" | 22" | | | |
| #4 | 22" | 22" | 30" | 22" | 30" | | | |
| #5 | 29" | 29" | 36" | 29" | 36" | | | |

-ONSTRUCTION

| R | evisions: | | | |
|-------|-------------------------------------|---|--|-------------------------------------|
| # | DATE | DE | SCRIPTION OF CHA | NGE |
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| | | | | GN-2.00 |

FOUNDATION PLAN (STANDARD) 01

ALL OPTIONS

CMU WALL PLAN (STANDARD) (02) ALL OPTIONS

LEGEND

| Revisions: # DATE DESCRIPTION OF CHANGE Image: Contract of the co | | | | | | | |
|---|---------------------------|--|--|--|--|---|------------------------------------|
| Revisions: # DATE DESCRIPTION OF CHANGE UTILIZED IN ANY FORM WITHOUT PRIOR WRITTEN AUTHORIZATION OF "COBALT". THIS DRAWING IS A CONCEPTUAL PLAN AND IS NOT INTENDED TO BE USED FOR PERMITTING PURPOSES. STE SPECIFIC PLAN NOR SHALL IT BE USED FOR PERMITTING PURPOSES. STE SPECIFIC SHALL BE PERPARED ON A CASE SHOUGH SHOULD BE SPECIFIC SHALL BE PERPARED ON A CASE SHOULD SHOULD INTENT IS PROHIBITED. INTENT IS PROHIBITED. INTENT IS PROHIBITED. | | | | | | | |
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| AW PROJECT #: SCALE: ¼"=1'-0" | T IN SH SF B/ | THIS DRAW TENDED TO HALL IT BE PECIFICS S ASIS AND A | ING IS A CO D BE USED USED FOR HALL BE P NY USE BE INTENT | ONCI AS / PEF REP EYON IS P | EPTUAL PLA A SITE SPEC MITTING PL ARED ON A ND THE AFO ROHIBITED. | N AND IS I CIFIC PLAN JRPOSES. CASE-BY-0 REMENTIC | NOT NOR SITE CASE DNED |
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CEILING JOIST PLAN (STANDARD) 01)

ALL OPTIONS

 $\left(02 \right)$

ALL OPTIONS

| MARK | HAN MEMBER SIZE | IGER SCHEDULE | NUMBER | | | LEGEND |
|------------|----------------------|--|-----------------|--|---|---|
| | 2X6 | LUS26 OR LUC2 | 26Z CONCEALED | _ | | - FULL HEIGHT BLOCKING |
| (2) | (2) 2X6 | LUS26-2 OR HUC | 26-2 CONCEALED | _ | | - TYPICAL STRONG BACK SEE |
| (3) | 2X8 | LUS28 OR LUC2 | 28Z CONCEALED | | | DETL. #4 ON DWG. SD-2.00 |
| | (2) 2X8 | LUS28-2 OR HUC | 28-2 CONCEALED | - | | PONY WALL TO ROOF DIAPHRAGM |
| (5) | 2X10 | LUS210 OR LUC2 | 210Z CONCEALED | | - | |
| (6) | (2) 2X10 | HUS210-2 OR HUC | 210-2 CONCEALED | | • | BRACE POINT |
| (7) | 2X12 | HUS210 OR LUC? | 210Z CONCEALED | | | BW - BRACE TO WALL BJ - BRACE TO JOIST |
| | SOME MO | DELS MAY NOT BE U | SED | | | BB - BRACE TO BEAM |
| BW | SOME MO (SEE FRAM | DELS MAY NOT BE UNITED DELS MAY NOT BE UNITED STATES AND STATES AN | SED TION) | C 1 2 1 2 1 2 3 4 5 5 7 1 2 3 4 7 7 1 2 3 4 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 | EILING FR ALL CEIL O.C. (U.N ATTIC AC 350LB C HEADER F HEADER SPECIFI STUD PACI I. (4) 2X S SOLID S COOF FRAM RIDGES ALL HIPS ALL HIPS ALL RAF ALL RAF ALL RAF C ALL RAF C ALL RAF C ALL RAF C ALL RAF C ALL STU EDGE & SECTION C AROUNE EDGE & SECTION C AROUNE EDGE & SECTION C AROUNE EDGE & SECTION C ALL STU ETC. SH DIRECTL | DD - DRACE TO BEAM BS - BRACE TO STRONGBACK NOTES AMING: ING JOISTS SHALL BE 2X6 #2 SYP 16" J.O.) CCESS SHALL BE NO LESS THAN APACITY FOLDING STAIR SYSTEM. RAMING: TO DETAIL 1 ON SD-1.00 FOR ALL REQUIREMENTS. (UNLESS IED ON PLAN) K FRAMING: TUD PACK UNDER EACH END OF 2X SAWN BEAM. MING: TO BE 2x10 #2 SYP S TO BE 2x8 #2 SYP TERS TO BE 2x6 #2 SYP 16"O.C. (UNO) HEATHING: ¹ %2" APA RATED DD W/ 0.120 X 3" RING SHANK NAILS, 4" D PERIMETER OF DIAPHRAGM. 6" 6" FIELD NAIL PATTERN. PER N R803 NDERLAYMENT SHALL COMPLY WITH .1 & TABLE R905.1.1.1 DS, FLOOR JOISTS, RAFTERS, TRUSS, ALL VERTICALLY ALIGN AND BEAR .Y ON EACH OTHER. DESCRIPTION OF CHANGE |
| | | | | | | |
| BW | B | S | | | | |
| ° | • | | | | | |
| | | | | ALL F | RIGHTS RESERVE | D. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR M WITHOUT PRIOR WRITTEN AUTHORIZATION OF "CORALT" |
| | | | | | / I UNI | CODALL . |
| | | | | | | |
| ВW | | _SLOPE | | T | HIS DRAWI | NG IS A CONCEPTUAL PLAN AND IS NOT |
| | | | | SH | | JSED FOR PERMITTING PURPOSES. SITE |
| | | | | SF B/ | YECIFICS SH ASIS AND AN | HALL BE PREPARED ON A CASE-BY-CASE |
| | BW | | | | | INTENT IS PROHIBITED. |
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| DRAWN BY: | AW | CHECKED BY: | ССН |
|------------|----|-------------|------------|
| PROJECT #: | | SCALE: | 1⁄4"=1'-0" |
| DATE: | | | S-1.00 |

| k | KING AND JACK STUD SCHEDULE | | | | |
|--------------------|---|--|--|--|--|
| OPENING SPAN | KING/JACK STUDS REQUIRED PER OPENING SPAN | | | | |
| 4' OR LESS | (1) KING & (1) JACK STUD (EACH SIDE) | | | | |
| 4' TO 6' OPENING | (2) KING & (2) JACK STUD (EACH SIDE) | | | | |
| 6' TO 8' OPENING | (3) KING & (2) JACK STUD (EACH SIDE) | | | | |
| 8' TO 12' OPENING | (4) KING & (3) JACK STUD (EACH SIDE) | | | | |
| 12' TO 16' OPENING | (5) KING & (4) JACK STUD (EACH SIDE) | | | | |

SINGLE STORY HEADER SCHEDULE

| ALL H | EADERS ARE DROP BEA | MS, U.N.O | GRADE # 2 LUMBER | | | |
|-------|---------------------|-----------------|--|--|--|--|
| TYPE | OPENING SPAN | WALL THICKNE | HEADER REQ'D SS | | | |
| 1 | 3' OR LESS | 3.5" | TWO 2"x8" #2 SYP w/ 1/2" PLYWD FLITCH | | | |
| 2 | 3' OR LESS | 5.5" | THREE 2"x8" #2 SYP w/ 1/2" PLYWD FLITCHS | | | |
| 3 | 3' TO 5' | 3.5" | TWO 2"x10" # 2 SYP w/ 1/2" PLYWD FLITCH | | | |
| 4 | 3' TO 5' | 5.5" | THREE 2"x10" #2SYP w/ 1/2" PLYWD FLITCHS | | | |
| 5 | 5' TO 8' | 3.5" | TWO 2"x12" #2 SYP w/ 1/2" PLYWD FLITCH | | | |
| 6 | 5' TO 8' | 5.5" | THREE 2"x12" #2 SYP w/ 1/2" PLYWD FLITCHS | | | |
| | | | | | | |

OPENINGS GREATER THAN 6'-1" SHALL BE AS INDICATED ON PLAN DRAWINGS NOTES:

THIS TABLE USES EITHER 0.25" DEFLECTION OR L/240 WHICHEVER IS LESS 1

THIS TABLE ACCOUNTS FOR ONLY ROOF AND CEILING LOADING 2.

3. FOR GENERIC 1 STORY FRAMING 4. FOR 2-STORY DWELLINGS USE 2X12 HEADERS ON FIRST FLOOR LIVING SPACE (UNO)

ALL NON-LOAD BEARING HEADERS SHALL BE 2X6 #2 SYP W/ $\frac{1}{2}$ " PLYWOOD FLITCH. *

INTEREIOR HEADER SCHEDULES 1

(3) BEAM TO FILLED CMU WALL DETAIL

SCHEDULE

CLIP & STRAP SCHEDULE

| CONNECTION LOCATION | CLIP / STRAP |
|--------------------------------|----------------------|
| STUD TO BOTTOM PLATE (SLAB) | SIMPSON H2.5A 1 |
| STUD TO STRINGER/BEAM (PILING) | SIMPSON LSTA36 23 |
| STUD TO STUD BETWEEN LEVELS | SIMPSON LSTA36 23 |
| HEADER STRAPS (8' PLATE) | SIMPSON LSTA36 13 |
| HEADER STRAPS (9' PLATE) | SIMPSON CS16 (48") 1 |
| 2X RAFTER TO TOP PLATE/ BEAM | SIMPSON H2.5A 1 |
| RAFTER TO RAFTER @ RIDGE | SIMPSON LSTA18 1 |
| RAFTER TO RAFTER @ HIP | SIMPSON LSTA18 1 |
| ROOF TRUSS TO STUD | SIMPSON HTS30C 1 |
| RAFTER TO TOP PLATE | (2) SIMPSON H2.5A 1 |
| STUD TO TOP PLATE | (2) SIMPSON H2.5A 1 |
| | |

1. FILL ALL NAIL HOLES U.N.O.

- 2. STUD STRAPS SHALL TERMINATE AT A BEAM OR STRINGER (NEVER AT BAND JOIST) & EXTEND PAST RIM JOIST BY A MINIMUM OF 10".
- 3. SIMPSON CS16 (OF EQUAL LENGTH AND FASTENERS) MAY BE SUBSTITUTED FOR SIMPSON LSTA
- 4. FASTENERS & QUANTITY PER MANUFACTURER'S
- INSTALLATION INSTRUCTIONS.
- 5. SOME CLIPS & STRAPS LISTED MAY NOT APPLY TO EVERY PROJECT.

| R | evisions: | | |
|---|-----------|-----------------------|--|
| # | DATE | DESCRIPTION OF CHANGE | |
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| DRAWN BY: | AW | CHECKED BY: | ССН |
|------------|----|-------------|---------|
| PROJECT #: | | SCALE: | N.T.S. |
| | | | SD-1.00 |

| LOCATION | SQUARE FOOTAGE | |
|-------------|----------------|--|
| FLOOR PLAN | 1,195 S.F. | |
| FRONT PORCH | 54 S.F. | |
| REAR PORCH | 25 S.F. | |
| | • | |

| MARK | QTY | DESCRIPTIONS | |
|------|-----|---------------|--|
| Α | 6 | 3'-0" X 5'-0" | |
| В | 1 | 3'-0" X 3'-0" | |

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| GHT | - |
|-----|---|
| | |
| | |

DATE:

SCALE:

RESPONSIBILITIES:

- GENERAL CONTRACTOR IS RESPONSIBLE FOR VERIFYING STRUCTURAL DRAWINGS & ARCHITECTURAL DRAWINGS ARE IN AGREEMENT. ANY DISCOURSE BETWEEN THE TWO SETS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER & ARCHITECT FOR RESOLUTION.
- GENERAL CONTRACTOR IS RESPONSIBLE FOR VERIFYING STRUCTURAL DRAWINGS TO CONFIRM THE AVAILABILITY OF ALL REQUIRED DETAILS. IF ANY REQUIRED INFORMATION IS NOT LISTED IN THE STRUCTURAL DRAWINGS, GENERAL CONTRACTOR IS REQUIRED TO CONTACT ENGINEER OF RECORD FOR PARTICULAR INFORMATION.
- 3. GENERAL CONTRACTOR IS RESPONSIBLE FOR ENSURING CONSTRUCTION ADHERES TO FEMA FLOOD PLAIN, LOCAL MUNICIPALITY, (FFE) FINISH FLOOR ELEVATION REQUIREMENTS AND/OR DISASTER RECOVERY CONSTRUCTION GUIDELINES.
- 4. GENERAL CONTRACTOR IS RESPONSIBLE FOR COMPLIANCE WITH ALL LOCAL MUNICIPALITY CODE REQUIREMENTS AND/OR DISASTER RECOVERY CONSTRUCTION GUIDELINES.
- COBALT ENGINEERING IS NOT RESPONSIBLE FOR DESIGN FLOOD ELEVATION OR ANY REQUIREMENT EXCEEDING THE BASE FLOOD ELEVATION AS SHOWN ON THE PROVIDED ELEVATION CERTIFICATE.
- COBALT ENGINEERING & INSPECTIONS LLC. IS NOT RESPONSIBLE FOR THE PERFORMANCE OF FOUNDATION AS A RESULT OF THE BEHAVIOR OF THE SUPPORTING SOIL AND/OR DIFFERENTIAL SETTLEMENT DUE TO SEASONAL CHANGES SUCH AS DROUGHT, EXTENSIVE RAIN AND OTHER DRASTIC CLIMATE CHANGES

CONCRETE:

- 1. UNLESS OTHERWISE NOTED, ALL CONCRETE FOUNDATION WALLS AND SLABS ON GRADE SHALL BE 3,000 PSI (28 DAY COMPRESSION STRENGTH) CONCRETE, PLACE CONCRETE SLABS ON 4" OF COMPACTED LOW P.I SAND FILL. ALL SLABS UNDER INTERIOR FINISHED AND HEATED LIVING SPACES SHALL BE PLACED ON 6 MIL POLYETHYLENE VAPOR BARRIER WITH A MINIMUM OF 6" LAPPED JOINTS. ALL REBAR LAP SPACING LENGTHS SHALL BE MINIMUM 50 TIMES THE BAR SIZE.
- PROVIDE ¹/₂" EXPANSION JOINT MATERIAL BETWEEN ALL CONCRETE SLABS ABUTTING CONCRETE OR MASONRY WALLS OCCURRING IN EXTERIOR OR UNHEATED SPACES OR AREAS.
- CONCRETE FOR ALL BASEMENT WALLS, FOUNDATION WALLS, PORCHES, WALKS, PATIOS, STEPS, GARAGE, CARPORT FLOOR SLABS AND DRIVEWAYS SHALL BE AIR-ENTRAINED.
- 4. ALL REINFORCEMENT STEEL SHALL MEET ASTM A615 GRADE 60 SPECIFICATIONS.
- REBAR COVER: PROVIDE THE FOLLOWING CONCRETE PROTECTIVE COVERINGS FOR REINFORCEMENT, UNLESS NOTED OTHERWISE.
- 3" FOR ALL CONCRETE DEPOSITED DIRECTLY AGAINST THE GROUND. 2" FOR ALL FORMED CONCRETE EXPOSED TO WEATHER OR IN CONTACT WITH THE В. GROUND. (UNLESS NOTED OTHERWISE)
- 6. DEVELOPMENTAL LENGTH (DL) SHALL BE 50 TIMES THE DIAMETER OF THE REBAR

RESPONSIBILITIES:

A. ALL (EARTHWORK, EXCAVATIONS, STRUCTURAL FILL, COMPACTION, ETC) SHALL BE DONE IN ACCORDANCE WITH GEO-TECHNICAL REPORT.

1. EXCAVATING

- A ALL EXCAVATION SHALL COMPLY WITH OSHA STANDARDS
- B. DEMOLISH AND REMOVE ALL OBSTRUCTIONS AS REQUIRED. AREAS TO RECEIVE FOUNDATION SHALL BE STRIPPED TO REMOVE ALL ORGANIC MATERIAL AND CONTAMINATED OR SOFT SOIL. DISPOSAL OF ALL DEBRIS FROM DEMOLITION AND STRIPPING OPERATIONS SHALL BE AS SPECIFIED BY OWNER.
- C. CARE SHALL BE TAKEN TO NOT OVER EXCAVATE BELOW BOTTOM OF FOUNDATION. ANY OVER EXCAVATION REQUIRED FOR REMOVAL OF THE EXISTING FILL SHALL BE BACKFILLED PER SECTION 2.A OR 2.B
- D. ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY PROOF-ROLLING PER TX DOT ITEM 216 TO DETERMINE THAT ALL LOOSE, SOFT, OR OTHERWISE UNDESIRABLE MATERIALS ARE REMOVED. IF AN AREA OF UNDESIRABLE MATERIAL IS DISCOVERED AT THE BOTTOM OF THE EXCAVATION, IT SHALL BE REMOVED AND REPLACED WITH COMPACTED BACKFILL PER SECTION 2.A. OR 2.B.
- E. THE UPPER 6" OF EXPOSED SOILS SHALL BE COMPACTED TO AT LEAST 95% OF MAX. DRY DENSITY DETERMINED BY MODIFIED PROCTOR TEST (ASTM D1557).
- F. WHERE SOIL CONDITIONS PERMIT, FOUNDATIONS BELOW GRADE MAY BE EARTH FORMED UNLESS OTHERWISE NOTED.

2. ENGINEERED FILL (CLAY)

- A. CLAY STRUCTURE FILL SHALL BE SANDY CLAY WITH LIQUID LIMIT OF LESS THAN 35 AND PLASTICITY INDEX (PI) BETWEEN 8 AND 20.
- CLAY SHALL BE MOISTURE CONDITIONED WITHIN 2% OF OPTIMUM MOISTURE CONTENT AND COMPACTED TO AT LEAST 95 % OF THE MAX DRY DENSITY DETERMINED BY THE MODIFIED PROCTOR TEST (ASTM D1557) WITH 8" MAX. LOOSE LIFTS.
- FOR COMPACTION BY MANUALLY-GUIDED POWER COMPACTORS, STRUCTURAL FILL SHALL BE PLACED IN LIFTS OF 6" MAXIMUM LOOSE THICKNESS.

NOTES ON PRESSURE TREATED LUMBER

ALL WOOD MEMBERS (INCLUDING PLYWOOD SHEATHING & ALL WOOD BASED MATERIALS) IN CONTACT WITH CONCRETE, OR EXPOSED TO WEATHER, MOISTURE OR WITHIN 18" OF THE GROUND (SUCH AS PORCH & BALCONY FRAMING) SHALL BE PRESSURE- TREATED.

GALVANIZED

FASTENERS (AND OTHER METAL PRODUCTS) FOR USE WITH WOOD TREATED WITH ACQ PRESERVATIVES INCLUDE: HOT-DIP GALVANIZED (THE MINIMUM STANDARD) THE MINIMUM HOT-DIP GALVANIZED REQUIREMENT FOR USE WITH TREATED WOOD SHOULD CONFORM TO THE FOLLOWING ASTM STANDARDS: ASTM- A153 (FOR HOT-DIP FASTENER PRODUCTS) AND ASTM-A653 (COATING DESIGNATION g-185 FOR HOT-DIP CONNECTOR AND SHEET PRODUCTS).

STAINLESS STEEL STAINLESS STEEL FASTENERS AND CONNECTORS ARE REQUIRED FOR PERMANENT WOOD FOUNDATIONS BELOW GRADE AND ARE RECOMMENDED FOR USE WITH TREATED WOOD IN OTHER SEVERE EXTERIOR APPLICATIONS SUCH AS SWIMMING POOLS, SALT WATER EXPOSURE, ETC. - TYPE 304 AND 316 ARE THE RECOMMENDED GRADES TO US

ALUMINUM SHOULD NOT BE USED IN DIRECT CONTACT WITH PRODUCTS TREATED WITH ACQ PRESERVATIVE SPACER MATERIALS OR OTHER PHYSICAL BARRIERS ARE RECOMMENDED TO PREVENT DIRECT CONTACT OF ACQ TREATED WOOD WITH ALUMINUM PRODUCTS.

STRUCTURAL:

 IF TRUSSES ARE SPECIFIED ON THE PLANS, THE TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND/OR STRESS AND LOAD CALCULATIONS (DIAGRAMS) FOR CONTRACTORS APPROVAL PRIOR TO CONSTRUCTION. DRAWINGS SHALL BEAR SEAL OF THE REGISTERED ENGINEER IN THE STATE IN WHICH THE STRUCTURE IS BUILT

MISC. BOLTS AND THREADED FASTENERS

- A. SPECIFICATION
- B. DESIGN
- C. INSTALLATION

4. BOLTED CONNECTIONS SHALL BE KNURLED OR SPOT-WELDED TO PREVENT BACK-OUT. 1603.1.4 WIND DESIGN DATA:

THE FOLLOWING INFORMATION RELATED TO WIND LOADS SHALL BE SHOWN, REGARDLESS OF WHETHER WIND LOADS GOVERN THE DESIGN OF THE LATERAL FORCE-RESISTING SYSTEM OF THE STRUCTURE

- RISK CATEGORY.
- UTILIZED.

| Notes: |
|---|
| Values are ultimate wind speeds in miles p ft (10m) above ground egory. |
| Linear interpolation is permitted. |
| 3. Islands and coast last contour shall use contour of the coastal |
| Mountainous terra promontories, and spec shall be examined for tions. |
| 5. Wind speeds correr a 7% probability of ex (Annual Exceedance Pro MRI = 700 years). |

CURRENTLY, THE PRODUCT COMMONLY USED FOR PRESSURE TREATMENT IS ALKALINE COPPER QUATERNARY (ACQ). THIS MATERIAL IS EXTREMELY CORROSIVE. ONLY HOT- DIPPED

ANCHOR BOLTS, THRU BOLTS, NAILS, OR OTHER CORROSIVE-RESISTANT FASTENERS, SHALL BE USED WITH ACQ-TREATED LUMBER. FASTENER MANUFACTURER OR SUPPLIER SHALL BE CONSULTED ON THE SUITABILITY OF GALVANIZED FASTENERS FOR USE WITH TREATED LUMBER.

1. BOLTS SHALL CONFIRM TO ASTM A307 GRADE A. CARBON STEEL EXTERNALLY THREADEDFASTENERS, U.N.O.

2. BOLTS AND NUTS SHALL BE HEX HEAD ASTM A307 AND CONFORM TO ANSI STANDARDS B18.2.1 AND B18.2.1 AS WELL AS ASTM MATERIAL STANDARDS ASTM 307. 3. WASHERS SHALL BE CIRCULAR, FLAT AND SMOOTH IN CONFORMANCE WITH THE

REQUIREMENTS OF TYPE A WASHERS IN ANSI STANDARD B23.1.

1. THE MINIMUM BOLT DIAMETER SHALL BE $\frac{1}{2}$ INCH AND BE BEARING TYPE CONNECTION USING STANDARD HOLES WITH THREADS EXCLUDED FROM THE SHEAR PLANE, U.N.O.

1. A307 BOLTS SHALL BE TIGHTENED PER TURN-OF-NUT BOLTING METHOD. THE TURN-OF-NUT BOLTING METHOD CONDITION DEFINED AS THE TIGHTNESS ATTAINED BY THE EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH. THEN BACKED OFF 2/3 TURN. (PER AISC MANUEL OF STEEL CONSTRUCTION, LOAD & RESISTANCE FACTOR 3RD EDITION 8.2.1) DO NOT OVERTIGHTEN BOLTS AS TO DAMAGE THE WOOD ELEMENTS.

2. BOLTS AND NUTS SHALL BE WELL LUBRICATED AT TIME OF INSTALLATION. DRY. CORRODED BOLTS WILL NOT BE ALLOWED.

ALL BOLTS SHALL BE NEW AND NOT REUSED.

BASIC DESIGN WIND SPEED, V, MILES PER HOUR AND ALLOWABLE STRESS DESIGN WIND SPEED, VASD, AS DETERMINED IN ACCORDANCE WITH SECTION 1609.3.1.

WIND EXPOSURE. APPLICABLE WIND DIRECTION IF MORE THAN ONE WIND EXPOSURE IS

APPLICABLE INTERNAL PRESSURE COEFFICIENT

DESIGN WIND PRESSURES TO BE USED FOR EXTERIOR COMPONENT AND CLADDING MATERIALS NOT SPECIFICALLY DESIGNED BY THE REGISTERED DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN OF THE STRUCTURE, PSF (KN/M2).

GREATER WIND ZONE. OR

OPENING BEING COVERED

CODE:

ZONE LEGEND

ZONE 5 AS INDICATED ARE ZONE 4

WINDOWS, DOORS AND WALLS THE ALPHA VALUE DEFINES THE SIZE OF ZONE 5 (MEASURED FROM ANY OUTSIDE CORNER)

VERIFY WITH PLANS

| | | COMPONE | NIL |
|------|---------|---------|-----|
| ZONE | DOOR | WINDOW | WAI |
| 1 | | | |
| 2 | | | |
| 3 | | | |
| 4 | +32/-34 | +34/-36 | |
| 5 | +32/-41 | +34/-45 | |
| ALL | | | |

EXPOSURE = C

INTERNAL PRESSURE COEFFICIENT = 0.18 PRESSURE AREA 10 SQ. FT.

DESIGN LOADS:

2. FLOOR LIVE LOADS

| USE | LOAD (|
|--------------------|--------|
| EXTERIOR BALCONIES | 40 |
| DECKS | 40 |
| FIRE ESCAPES | 40 |
| STAIRS/RAMPS | 40 |
| | 25 |

- FLATTER.
- POINT ALONG THE TOP.

4. DEAD LOAD

| USE | LOAD (PSF |
|-------|-----------|
| ROOF | 10 |
| DECKS | 10 |
| FLOOR | 10 |
| WALLS | 11 |

| | MINIMU |
|------------------|----------------|
| 1 STORY HOUSE | WALL TYPE |
| | EXTERIOR WALLS |
| | INTERIOR WALLS |
| | PLUMBING WALLS |
| | |

NOTES:

DIMENSIONS: BEGINS.

FLORIDA WIND BORNE DEBRIS PROTECTION **TABLE OF CONTENTS** XX-XXXX-GN-1.00 GENERAL NOTES ' THIS STRUCTURE IS IN A WIND BORNE DEBRIS PROTECTION AREAS. IT IS WITHIN 1 MILE OF THE XX-XXXX-GN-2.00 **GENERAL NOTES 2** COASTAL MEAN HIGH WATER LINE WHERE THE ULTIMATE WIND SPEED, Vult. AND IS 130MPH OR XX-XXXX-S-1.00 FOUNDATION PLAN XX-XXXX-S-2.00 STRINGER PLAN XX-XXXX-S-3.00 FLOOR JOIST PLAN IN AN AREA WHERE THE ULTIMATE DESIGN WIND SPEED, Vult. IS 140MPH OR GREATER. XX-XXXX-S-4.00 CEILING JOIST PLAN WIND BORNE DEBRIS PROTECTIONS SHALL MATCH OR EXCEED THE DESIGN PRESSURE FOR THE XX-XXXX-S-5.00 RAFTER PLAN XX-XXXX-S-6.00 SHEARWALL & HOLD-DOWN PLAN STANDARD DETAILS 1 XX-XXXX-SD-1.00 **CODES & DESIGN LOADS** XX-XXXX-SD-2.00 **STANDARD DETAILS 2** XX-XXXX-SD-3.00 **STANDARD DETAILS 3** XX-XXXX-SD-4.00 STANDARD DETAILS 4 FLORIDA BUILDING CODE- 2020 (CITY OF FORT PIERCE REQUIREMENTS) XX-XXXX-SD-5.00 **STANDARD DETAILS 5** LEGEND (MIN.) DESIGN PRESSURE FOR WINDSTORM COMPLIANCE) ALL AREAS NOT DESIGNATED AS TYPICAL DETAIL OR SECTION CALLOUT DETAIL PAGE (XX/XX-X.XX) **ABBREVIATIONS** $\alpha = 3'-0"$ SHOULD BE 10% OF SHORTEST WALL ANTHONY POWER BEAM APB APP. ANTHONY POWER PRESERVED COMPONENT DESIGN PRESSURE (PSF) CANT CANTILEVER CONT. CONTINUOUS LL COVERING GARAGE DOOR ROOF DWG. DRAWING +19/-30 --------FND. FOUNDATION +19/-53 --------F.V. FIELD VERIFY +19/-79 ----____ HDR. HEADER PLCS PLACES ____ ____ ----SIM. SIMILAR ____ ----____ S.F. STEEL FLITCH +25/-27 +28/-34 ____ SOUTHERN YELLOW PINE SYP. TRPL TRIPLE TYP. TYPICAL NOTES 1. ROOF LIVE LOADS 20 PSF (SUBJECT TO SLOPE & TRIBUTARY AREA REDUCTION FACTORS) 1. ALL SPECIFICATIONS ARE MINIMUM (PSF) USE LOAD (PSF REQUIREMENTS. SPECIFICATIONS MAY BE SLEEPING ROOMS 30 INCREASED TO MEET ARCHITECTURAL OR CONSTRUCTION PREFERENCES. **OTHER ROOMS** 40 ATTIC W/STORAGE 20 (b) THIS PLAN IS INTENDED TO BE PLOTTED AT ATTIC W/O STORAGE 10 (b) 24"X36". IF PLOTTED AT ANY OTHER SIZE IT 250(d) GARAGE N/A(a) GUARDRAILS & HANDRAILS | WILL NOT BE TO SCALE. THIS INCLUDES, BU (a) ELEVATED GARAGE FLOORS SHALL BE CAPABLE OF SUPPORTING A NOT LIMITED TO DIMENSIONS & ENGINEERS 2,000-POUND LOAD APPLIED OVER A 20-SQUARE-INCH AREA. SEAL. (b) NO STORAGE LOAD IS REQUIRED WITH ROOF SLOPES OF 3 IN 12, OR Revisions: (c) INDIVIDUAL STAIR TREADS SHALL BE DESIGNED FOR THE DATE DESCRIPTION OF CHANGE # UNIFORMLY DISTRIBUTED LIVE LOAD OR A 300-POUNDS CONCENTRATED LOAD ACTING OVER AN AREA OF 4 SQUARE INCHES, WHICHEVER PRODUCES THE GREATER STRESSES (d) A SINGLE CONCENTRATED LOAD APPLIED IN ANY DIRECTION @ ANY 3. WIND LOADS (AMERICAN SOCIETY OF CIVIL ENGINEERS, 7-16) ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR UTILIZED IN ANY FORM WITHOUT PRIOR WRITTEN AUTHORIZATION OF "COBALT" BASIC WIND DESIGN VELOCITY 160 MPH (ULTIMATE WIND SPEED) RISK CATEGORY: II THIS DRAWING IS A CONCEPTUAL PLAN AND IS NOT INTENDED TO BE USED AS A SITE SPECIFIC PLAN NOR SHALL IT BE USED FOR PERMITTING PURPOSES. SITE SPECIFICS SHALL BE PREPARED ON A CASE-BY-CASE BASIS AND ANY USE BEYOND THE AFOREMENTIONED INTENT IS PROHIBITED. IM WALL FRAMING REQUIREMENTS ANCHOR BOLT REQUIREMENTS MATERIAL & SPACING 5/8"X10" J-BOLTS W/ 3" SQ. X 2X6 SYP #2 @ 16" O.C. .125"THK. WASHER @ 32" O.C. 2X4 SYP #2 @ 16" O.C. 2X6 SYP #2 @ 16" O.C. 1. WALL SHEATHING SHALL BE AS INDICATED ON SHEARWALL & HOLDOWN PLAN. EXTERIOR COVERINGS SHALL CONFORM TO FBC INSTALLATION INSTRUCTIONS. 1. DRAWING DIMENSIONS GOVERN OVER SCALE. VERIFY ALL ROUGH OPENING DIMENSIONS FOR SELECTED DOORS, WINDOWS AND MECHANICAL REQUIREMENTS BEFORE CONSTRUCTION FASTENER CORROSION RESISTANCE FINISH HOT DIPPED GALV. (MIN.) ELECTRO GALVANIZED (MIN.) EPOXY COATED (MIN.)

DRAWING LIST

MOISTURE EXPOSURE

EXTERIOR ENCLOSED BUT VENTILATED

AIR CONDITIONED SPACE

DRAWN BY

PROJECT #:

GN-1.00

CCH N.T.S.

AW CHECKED BY

SCALE:

| NAILING | SCHEDULE | | | |
|---|---------------------------|------------------------|---------------------|--|
| JOINT DESCRIPTION | NUMBER OF COMMON NAILS | NUMBER OF BOX NAILS | NAIL SPACING | |
| WALL FRAM | MING | | | |
| Top plate to top plate (face-nailed) | 2-16d | 2-16d | per foot | |
| Top plates at intersections (face-nailed) | 4-16d | 5-16d | joints - each side | |
| Stud to stud (face-nailed) | 2-16d | 2-16d | 24"o.c. | |
| Header to header (face-nailed) | 16d | 16d | 16"o.c. along edges | |
| Top or bottom plate to stud (end-nailed) | 1 -16d | 1 -40d | per stud | |
| Bottom plate to floor joist, bandjoist, endjoist or blocking (face-nail | ed) 2-16d | 2-16d | per foot | |
| FLOOR FRA | MING | | | |
| Joist to sill, top plate or girder (toe-nailed) | 4-8d | 4-10d | per joist | |
| Bridging to joist (toe-nailed) | 2-8d | 2-10d | each end | |
| Blocking to joist (toe-nailed) | 2-8d | 2-10d | each end | |
| Blocking to sill or top plate (toe-nailed) | 3-16d | 4-16d | each block | |
| Ledger strip to beam (face-nailed) | 3-16d | 4-16d | each joist | |
| Joist on ledger to beam (toe-nailed) | 3-8d | 3-10d | per joist | |
| Band joist to joist (end-nailed) | 3-16d | 4-16d | per joist | |
| Band joist to sill or top plate (toe-nailed) | 2-16d | 3-16d | per foot | |
| CEILING SHEATHING | | | | |
| Gypsum wallboard | 5d coolers | 5d coolers | 7"edge/10"field | |
| ROOF FRAMING | | | | |
| Rafter to top plate (toe-nailed) | 5 -8d | 5 -10d | per rafter | |
| Ceiling joist to top plate (toe-nailed) | 5 -8d | 5 -10d | per joist | |
| Ceiling joist to parallel rafter (face-nailed) | 7 -16d | 7 -40d | each lap | |
| Ceiling joist laps over partitions (face-nailed) | 7 -16d | 7 -40d | each lap | |
| Collar tie to rafter (face-nailed) | 3 -8d | 3 -10d | per tie | |
| Blocking to rafter (toe-nailed) | 2-8d | 2 -10d | each end | |
| Rim board to rafter (end-nailed) | 2-16d | 3 -16d | each end | |

| CONCRETE COVERAGE FOR CAST-IN-PLACE (NON PRE-STRESSED CONCRETE MEMBERS) | | | | |
|--|---|--|--------------------|--|
| CONCRETE EXPOSURE | MEMBER | REINFORCEMENT | SPECIFIED COVER | |
| CAST AGAINST AND PERMANENTLY IN CONTACT W/ GROUND | ALL | ALL | 3" | |
| EXPOSED TO WEATHER OR IN CONTACT WITH GROUND | A1.1 | NO. 6 THROUGH NO. 18 BARS 2" | | |
| | ALL | NO. 5, W31 / D31 WIRE & SMALLER | 1-1⁄2" | |
| | SLABS, | BS, NO. 14 & NO. 18 1-½" | | |
| NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND | WALLS | NO. 11 BARS & SMALLER | ³ ⁄4" | |
| | BEAMS, COLUMNS, PEDESTALS, & TENSION TIES | PRIMARY REINFORCEMENT, STIRRUPS, TIES, SPIRALS, & HOOPS | 1-1⁄2" | |

| | DEVELOPMENT LENGTH CHART | | | | | |
|-------------|----------------------------|---------------------------|-------------------|--------|-------|--|
| | SLAB/ | | WA | LLS | | |
| BAR SIZE | BAR OR LESS GREATER THAN 1 | | (NESS THAN 12" | HORIZ. | VERT. | |
| | ALL BARS | BOTTOM OTHER BARS BARS | | | | |
| #3 | 17" | 17" | 22" | 17" | 22" | |
| #4 | 22" | 22" | 30" | 22" | 30" | |
| #5 | 29" | 29" | 36" | 29" | 36" | |

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| DATE: | | | GN-2.00 | | |

LEGEND

X

TYPE X TYPE OF NOTCH SIZE OF PILING (P06= 6x6) FOR PILING DEPTH DETAIL #6 DWG. SD-1.00 (SEE UNLESS NOTED OTHERWISE ON PLAN) SIZE OF CMU BLOCK (C12= 12X12)

(SEE SECTIONS A & B THIS DWG. FOR PIER CONSTRUCTION SECTIONS) UNLESS NOTED OTHERWISE ON PLAN)

NOTES

CONCRETE HOLD-DOWNS: 1. SEE DRAWING S-6.00 FOR TYPE OF HOLD-DOWN REQUIRED.

2. SEE DRAWING S-1.00 FOR HOLD-DOWN INSTALLATION DETAIL. (REFER TO MANUFACTURERS INSTALLATION INSTRUCTIONS FOR ADDITIONAL REQ'S.)

| Revisions: | | | | | |
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| PROJECT #: | | SCALE: | 1/4" = 1'-0" |
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5-1.00

LEGEND

DESIGNATES 6X6 HOLD DOWN BLOCK.

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03 FLOOR JOIST PLAN (STANDARD) ALL OPTIONS

| In Construction I | | HA MEMPER SIZE | | | | LEG | END | |
|---|-----------|--------------------------------------|---|---------------------|---|--|--|---|
| Image: Contract of the second status and se | (5) | 2X10 | LUS210 OR LUC210Z CONCEALED | | | | | |
| Hidd Direct State | 6 | (2) 2X10 | HUS210-2 OR HUC210-2 CONCEALED | | | | | |
| SOME MODELS MAY NOT THE USED (SEE PLAN FOR LOCATION) NOTES Image: Second | H RECC | OT DIPPED GAL OMMENDED ON EXPO | L VANIZED OR STAINLESS STEEL EXTERIOR HANGERS WITH DIRECT SURE TO MOISTURE | | | | | |
| | SO | ME MODELS MA | AY NOT BE USED (SEE PLAN FOR LOCATION) | | | | | |
| NOTES FLOOR FRAMING: 1. HOUSE FLOOR JOISTS SHALL BE 2X10 #2 SYP (#) 19' C.C. (UNO.) 2. DECK FLOOR JOIST SHALL BE 2X10 #2 SYP (#) 19' C.C. (UNO.) 3. DOUBLE THE FLOOR JOIST WHERE WALLS ADOVE RUN PARALLEL TO FLOOR JOIST WHERE WALLS ADOVE RUN PARALLEL TO FLOOR JOIST SHALL BE 2X10 #2 SYP (#) 19' C.C. (UNO.) 3. FLOOR DECKINS: ½' APA RATED PLYWOOD 0.120 x3 TAULS, 5' APA RATED PLWOOD 0.120 x3 TAULS, 5' APA RATED PLWO | | | | | | | | |
| FLOOR FRAMING: 1. HOUSE FLOOR JOISTS SHALL BE 2X12 #2 SYP @ 10° O.C. (U.N.O.) 2. DECK FLOOR JOISTS SHALL BE 2X10 #2 SYP @ 10° O.C. (U.N.O.) 3. DOUBLE THE FLOOR JOIST WHERE WALLS ABOVE FUN PARALLEL TO FLOOR JOIST. 4. FLOOR DECKING: ½", APA RATED PLYWOD 0.120 & 3" NALS, 6" AROUND PRIMETER OP DIAPHRAGM. 6" EDDE & 12" FIELD NAL PATTERN, (SEE DETAIL #300K, SD-200). FRAMING: 1. ALL STUDS, FLOOR JOISTS RAFTERS, TRUSS, ETC. SHALL VERTICALLY ALIGN AND BEAR DIRECTLY ON EACH OTHER. Revisions: * * INTE DESTOR OF CHARCE * INTERDED TO BE USED AS A SITE SPECIFIC FLAN NOR SHALL THE USED FOR PERMITTING PURPOSES. SITE SPECIFICS SHALL BE PREPARED OF A CASEE-YCARE. * INTENDED TO BE USED AS A SITE SPECIFIC FLAN NOR SHALL THE USED FOR PERMITTING PURPOSES. SITE SPECIFICS AND ANY USE BERYND THE AFOREMENTIONED INTENDED TO BE USED AS A SITE SPECIFIC FLAN NOR SHALL THE USED FOR PERMITTING PURPOSES. SITE SPECIFICS AND ANY USE BERYND THE AFOREMENTIONED INTENT IS PROHIBITED. | | | | | | NO | TES | |
| PECKTUOR JOISTS SHALL BE 2X10 #2 SYP @ 16" O.C. (U.N.O.) DOUBLE THE FLOOR JOIST WHERE WALLS ADVE FUND PRAILLE TO FLOOR JOIST. FLOOR DECKING: 3/" APA RATED PLYWOOD 0.120 & 3" NALS, 6" APOUND PERMIETER OF DIAPHRARM, SP EDE 4.12" FIELD NALL PATTERN, ISEE DETAIL #3 JWG, SD-2.00). FRAMING: ALL STUDS, FLOOR JOISTS, RAFTERS, TRUSS, ETC, SHALL VERTICALLY, ALIGN AND BEAR DIRECTLY ON EACH OTHER. Revision: ALTE DATE DESCRIPTION OF DIANGE ALTE DATE DESCRIPTION OF DIANGE ALTE DESCRIPTION OF DIANGE THIS DRAWING IS A CONCEPTUAL PLAN AND IS NOT INTENDED TO BE USED AS A SITE SPECIFIC PLAN. SHALL THE USED FOR PERMITTIN PURPOSES. SITE SHALL THE USE DRAFT AROTHER PURPOSE SHALL SE PREFERED AROTHER PURPOSE SHALL SE PROFT AROTHE | | | | FLC 1. | OR FRAM HOUSE 16" O.C | 1ING: EFLOOR JOIS . (U.N.O.) | TS SHALL BE 2X | (12 #2 SYP @ |
| OUGLE THE FLOOR JOIST WHERE WALLS ABOVE RUN PARALLEL TO FLOOR JOIST. FLOOR JOIST WALLS, 37 APA RATED PLYWOOD OLAPARAM. 6* DOGE AT 12* FLEID NALL PATTERN. (SEE DETAIL #3 DWG. SD-2.00). FRAMINC: ALL STUDS, FLOOR JOISTS, RAFTERS, TRUSS, ETC. SHALL VERTICALLY ALLGN AND BEAR DIRECTLY ON EACH OTHER AND BEAR DIRECTLY ON EACH OTHER AND BEAR DIRECTLY ON EACH OTHER ALL GUIDERSCRUCE, NO WALL OF THE BOOMENTIANY & GROUPLESS ALL THE LED FOR PERMITTING PLANADO BEAR DIRECTLY ON EACH OTHER ALL GUIDERSCRUCE, NO WALL OF THE BOOMENTIANY & GROUPLESS ALL THE LED FOR PERMITTING PLANADO BEAR DIRECTLY ON EACH OTHER ALL GUIDERSCRUCE, NO WALL OF THE BOOMENTIANY & GROUPLESS THIS DRAWING IS A CONCEPTUAL PLANADO BEAR INTENDED TO BE USED AS AN TE SPECIFIC PLANINGR SHALL THE LED FOR PERMITTING PLANEDESS STELL STRUCE AND WALL DE PERPENDENCES SPECIFICS SHALL DE PERPENDESS STELL STRUCE AND WALL DE PERPENDENCES SPECIFICS SHALL DE PERPENDENCES SPECIFICS SHALL DE PERPENDENCES SPECIFICS SHALL DE PERPENDENCES STELL STRUCE AND WALL DE PERPENDENCES SPECIFICS SHALL DE PERPENDENCES SP | | | | 2. | DECK F 16" O.C | FLOOR JOISTS . (U.N.O.) | S SHALL BE 2X1 | 0 #2 SYP @ |
| 4. FLOOR DECKING: 32" APA RATE DE VIVOOD 0. 120 × 37 NAULS, 57 ARAONID PERIMETER OF DIAPHRAGM, 6" EDGE & 12" FIELD NAIL PATTERN (SEE DETAIL #3 DWG, SD-2.00). FRAMING: 1. ALL STUDS, FLOOR JOISTS, RAFTERS, TRUSS, ETC. SHALL VERTICALLY ALION AND BEAR DIRECTLY ON EACH OTHER. Revisions: # DATE DESCRIPTION OF CHANGE | | | | 3. | DOUBL ABOVE | E THE FLOOF RUN PARALL | R JOIST WHERE LEL TO FLOOR J | WALLS OIST. |
| FRAMING: 1. ALL STUDS, FLOOR JOISTS, RAFTERS, TRUSS, ETC, SHALL VERTICALLY ALLON AND BEAR DIRECTLY ON EACH OTHER. Revisions: ************************************ | | | | 4. | FLOOR 0.120 x DIAPHF PATTEI | DECKING: 3⁄4 3" NAILS, 6" A RAGM. 6" EDG RN. (SEE DET | " APA RATED PL AROUND PERIME E & 12" FIELD N AIL #3 DWG. SD | YWOOD ETER OF AIL -2.00) |
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| AL BIGHTS RESTURD. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR UTLIZED IN ANY FORM WITHOUT PRIOR WRITTEN AUTHORIZATION OF "COBALT". THIS DRAWING IS A CONCEPTUAL PLAN AND IS NOT INTENDED TO BE USED AS A SITE SPECIFIC PLAN NOR SHALL IT BE USED FOR PERPARED ON A CASE-BY-CASE BASIS AND ANY USE BEYOND THE AFOREMENTIONED INTENT IS PROHIBITED. | | | | R | evisions: DATE | DE | SCRIPTION OF CHAN | IGE |
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04

ALL OPTIONS

PROJECT #:

DATE:

S-4.00

1/4" = 1'-0"

LEGEND

BRACE POINT

BW - BRACE TO WALL BJ - BRACE TO JOIST BB - BRACE TO BEAM BS - BRACE TO STRONGBACK

NOTES

ROOF FRAMING: 1. RIDGES TO BE 2x10 #2 SYP

- 2. ALL HIPS TO BE 2x8 #2 SYP
- 3. ALL RAFTERS TO BE 2x6 #2 SYP 16"O.C. (UNO)
- ROOF SHEATHING: ¹⁹/₃₂" APA RATED PLYWOOD W/ 0.120 X 3" RING SHANK NAILS, 4" AROUND PERIMETER OF DIAPHRAGM. 6" EDGE & 6" FIELD NAIL PATTERN. PER SECTION R803
- 5. ROOF UNDERLAYMENT SHALL COMPLY WITH R905.1.1.1 & TABLE R905.1.1.1

FRAMING:

1. ALL STUDS, FLOOR JOISTS, RAFTERS, TRUSS, ETC. SHALL VERTICALLY ALIGN AND BEAR DIRECTLY ON EACH OTHER.

| Revisions: | | | | |
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| DATE: | | | S-5.00 |

SHEAR WALLS: ALL SHEARWALLS SHALL EXTEND TO ROOF DIAPHRAGM OR NEXT FLOOR DIAPHRAGM.

SW1: 19/32" APA RATED PLYWOOD SHEATHING 6" PERIMETER AND W/6" EDGE / 12" FIELD NAIL PATTERN USING 0.120 x 3" NAILS

 $\sqrt{SW4}$

SW4: 19/32" APA RATED PLYWOOD SHEATHING 4" PERIMETER AND W/4" EDGE / 12" FIELD NAIL PATTERN USING 0.120 x 3" NAILS

HOLD-DOWNS:

ALL HOLD-DOWNS SHALL BE SECURED TO MIN. DBL STUD OR GREATER PER MANUFACTURERS INSTRUCTIONS.

MSTC48B3 W/ (16) 10D NAILS IN BEAM & 1 (38) 10D NAILS IN STUD PACK PER MANUFACTURERS INSTRUCTIONS

MSTC66B3 W/ (18) 10D NAILS IN BEAM & (38) 10D NAILS IN STUD PACK PERMANUFACTURERS INSTRUCTIONS

HDU5-SDS2.5 USE (14) ¹/₄"X2¹/₂"SDS SCREWS 3 PER MANUFACTURERS INSTRUCTIONS (SEE HDU TO FLOOR BEAM CONNECTION DETAIL ON THIS DWG. FOR INSTALLATION TO FLOOR BEAM).

★ DRAG STRUT BEAM W/ SIMP. HGA10KT AT EACH END (SEE DETAIL S-4.00)

| R | evisions: | |
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SCALE:

HDU TO FLOOR BEAM CONNECTION DETAIL

6 TYPICAL EM

 Revisions:

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 DATE

 DESCRIPTION OF CHANGE

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| KING AND JACK STUD SCHEDULE | | | | |
|-----------------------------|---|--|--|--|
| OPENING SPAN | KING/JACK STUDS REQUIRED PER OPENING SPAN | | | |
| 4' OR LESS | (1) KING & (1) JACK STUD (EACH SIDE) | | | |
| 4' TO 6' OPENING | (2) KING & (2) JACK STUD (EACH SIDE) | | | |
| 6' TO 8' OPENING | (3) KING & (2) JACK STUD (EACH SIDE) | | | |
| 8' TO 12' OPENING | (4) KING & (3) JACK STUD (EACH SIDE) | | | |
| 12' TO 16' OPENING | (5) KING & (4) JACK STUD (EACH SIDE) | | | |

| BEND | MSTA3 |
|------|---------|
| BACK | SIDE C |
| PACK | (6" DO\ |
| STUD | MIN.) |
| | , |

(2) LSTA24 STRAPS

EACH SIDE OF BEAM

AS SHOWN, (4) TOTAL

(2) TOE-NAILS

STRINGERS PER

2

PLAN

| | SIN | GLE STOR | Y HEADER SCHEDULE | | | | | |
|--|--------------|-----------------|---|--|--|--|--|--|
| ALL HEADERS ARE DROP BEAMS, U.N.O GRADE # 2 LUMBER | | | | | | | | |
| TYPE | OPENING SPAN | WALL THICKNE | HEADER REQ'D SS | | | | | |
| 1 | 3' OR LESS | 3.5" | TWO 2"x8" #2 SYP w/ 1/2" PLYWD FLITCH | | | | | |
| 2 | 3' OR LESS | 5.5" | THREE 2"x8" #2 SYP w/ 1/2" PLYWD FLITCHS | | | | | |
| 3 | 3' TO 5' | 3.5" | TWO 2"x10" #2SYP w/ 1/2" PLYWD FLITCH | | | | | |
| 4 | 3' TO 5' | 5.5" | THREE 2"x10" #2SYP w/ 1/2" PLYWD FLITCHS | | | | | |
| 5 | 5' TO 8' | 3.5" | TWO 2"x12" #2 SYP w/ 1/2" PLYWD FLITCH | | | | | |
| 6 | 5' TO 8' | 5.5" | THREE 2"x12" #2 SYP w/ 1/2" PLYWD FLITCHS | | | | | |
| | | • | | | | | | |

OPENINGS GREATER THAN 6'-1" SHALL BE AS INDICATED ON PLAN DRAWINGS NOTES:

THIS TABLE USES EITHER 0.25" DEFLECTION OR L/240 WHICHEVER IS LESS

2

THIS TABLE ACCOUNTS FOR ONLY ROOF AND CEILING LOADING

FOR GENERIC 1 STORY FRAMING 3.

FOR 2-STORY DWELLINGS USE 2X12 HEADERS ON FIRST FLOOR LIVING SPACE (UNO) 4.

SLAB ON GRADE

TYPICAL DOOR/ WINDOW FRAMING DETAILS

| 2x6 RAIL CAP 2x4 TOP & BOTTOM RAILS ATTACHED TO GUARD POST W(2)8d THREADED NAILS OR (2)#8 SCREWS 2 ½" LONG ON INSIDE FACE 2x2 BALUSTERS @ 5" O.C. MAX. ATTACH TO TOP AND BOTTOM RAILS W(1)#8 WOOD SCREW OR (2)8d (0.135"0) THREADED NAILS W(1)#8 WOOD SCREW OR (2)8d (0.135"0) THREADED NAILS W(1)#8 WOOD SCREW OR (2)8d (0.135"0) THREADED NAILS W(1)#8 WOOD SCREW OR (2)8d (0.135"0) THREADED NAILS DO NOT NOTCH POST WIN. 4x4 POST @ 6'-0"O.C. MA ATTACHED TO DECK JOIST W/(2 ½" GALV. THRU-BOLTS. DO NOT NOTCH POST BLOCKING ON EACH SIDE OF POST DECK JOISTS PER PLAN (3) ½" GALV. THRU-BOLTS (H.D.G.). BAND JOIST PER PLAN | | | | | |
|--|---------------------------|--|---|---|--|
| N BLOCKING TOGETHER WITH (12) NAILS (0.131"x2-1/2") | | | | | |
| DECK RAIL INSTALLATION DETAIL | | | | | |
| | | | | | |
| | | | | | |
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| | R | evisions: DATE | DE | ESCRIPTION OF CH | ANGE |
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| | ALL I UTILIZ | RIGHTS RESERV ZED IN ANY FOF | ED. NO PART OF TH | IIS DOCUMENT MAY B WRITTEN AUTHORIZA | E REPRODUCED OR TION OF "COBALT". |
| | ר IN SI SI B/ | THIS DRAW TENDED TO HALL IT BE PECIFICS S ASIS AND A | ING IS A CONC D BE USED AS USED FOR PEI HALL BE PREF NY USE BEYO INTENT IS F | EPTUAL PLAN A A SITE SPECIFIC RMITTING PURP PARED ON A CAS ND THE AFOREM PROHIBITED. | ND IS NOT C PLAN NOR OSES. SITE SE-BY-CASE MENTIONED |
| | | | | | |
| | | | | | |
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| | | | | | |
| | | | | | |
| | | | | | |
| | PROJE | ECT #: | | SCALE: | N.T.S. |
| | DATE: | | | | SD-4.00 |

| | ç | 9 | 1 | 0 | 1 | 1 | 1 | 2 | 1 | 13 | | 14 |
|-----|----|-----|----|-----|----|-----|----|-----|----|-----|----|----|
| 10d | 8d | |
| 16 | 28 | 18 | 32 | 20 | 34 | 22 | 37 | 24 | 40 | 26 | 43 | |
| | | | | | | | | | | | | _ |

| | ç |) | | 10 | | 11 | 1 | 12 | 1 | 3 | | 14 |
|----|----|-----|----|-----|----|-----|----|-----|----|-----|----|----|
| 0d | 8d | 10d | 8d | |
| 20 | 34 | 24 | 38 | 28 | 42 | 30 | 46 | 32 | 50 | 35 | 53 | |

| | ç | 9 | | 10 | 1 | 1 | 1 | 2 | | 13 | 1 | 4 |
|----|----|-----|----|-----|----|-----|----|-----|----|-----|----|---|
| 0d | 8d | 10d | 8d | |
| 24 | 42 | 28 | 46 | 32 | 52 | 35 | 56 | 38 | 61 | 41 | 66 | |

FEMA BREAKAWAY WALL FASTENING TABLE

NOTES

- THIS DRAWING MAY NOT APPLY IF THE 1. STRUCTURE IS NOT IN A FLOOD ZONE.
- FEMA APPROVED FLOOD VENTS ARE 2. REQUIRED WHEN ENCLOSED SPACE IS BELOW B.F.E. IN AN "A" FLOOD ZONE.
- FEMA APPROVED BREAKAWAY WALLS OR 3. LOUVERS ARE REQUIRED WHEN ENCLOSED SPACE IS BELOW B.F.E. IN A "V" FLOOD ZONE. (EXCEPTION: UP TO 299 SQ. FT. MAY BE ENCLOSED W/O BREAKAWAY OR LOUVERED WALLS.)
- ALL MATERIALS BELOW B.F.E. SHALL BE 4. FLOOD DAMAGE RESISTANT MATERIAL, INCLUDING, BUT NOT LIMITED TO TREATED LUMBER. SEE FEMA TECHNICAL BULLETIN #2 FOR ADDITIONAL INFORMATION.

| R | Revisions: | | | | | | | | | |
|-------|--|-----------------------|--|--|--|--|--|--|--|--|
| # | DATE | DESCRIPTION OF CHANGE | | | | | | | | |
| | | | | | | | | | | |
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ALL BREAKAWAY WALLS DESIGNED USING THE PRESCRIPTIVE DESIGN METHOD SHALL BE DETAILED IN ACCORDANCE WITH THE FOLLOWING:

1. BREAKAWAY WALLS SHALL BE DESIGNED TO MEET ALL APPLICABLE LOCAL REQUIREMENTS AND BUILDING CODE REQUIREMENTS.

2. AS SHOWN IN BREAKAWAY WALL SECTION (THIS DWG.), WOOD-FRAMED AND STEEL STUD-FRAMED BREAKAWAY WALL PANELS SHALL NOT BE ATTACHED TO THE PILINGS OR OTHER VERTICAL FOUNDATION MEMBERS. ONLY THE TOPS AND BOTTOMS OF WALL PANELS SHALL BE CONNECTED TO PERMANENT 2X4 NAILER PLATES. HIGH-CAPAC ITY CONNECTORS SUCH AS BOLTS, LAG SCREWS, METAL STRAPS, OR HURRICANE FASTENERS (E.G., CLIPS OR STRAPS) SHALL

3. THE EXTERIOR SHEATHING ON BREAKAWAY WALL PANELS SHALL NEITHER OVERLAP NOR BE ATTACHED TO THE VERTICAL FOUNDATION MEMBERS.

4. BREAKAWAY WALL SHEATHING AND SIDING SHALL BE DISCONTINUOUS AT ELEVATED FLOOR BEAMS AND JOISTS; HORIZONTAL SEPARATION JOINTS SHALL BE PROVIDED TO PREVENT DAMAGE TO THE SHEATHING OR SIDING ABOVE THE FLOOR OF THE ELEVATED BUILDING.

5. UTILITIES, INCLUDING ELECTRICAL WIRING, BREAKER BOXES, POWER METERS, PLUMBING, CONDUITS, AND VENTILATION DUCTS, SHALL NOT BE PLACED IN OR ATTACHED TO BREAKAWAY WALL PANELS.

6. BREAKAWAY WALL PANELS SHALL BE POSITIONED SUCH THAT, ON FAILURE, THEY DO NOT COLLAPSE AGAINST CROSS-BRACING OR THREATEN OTHER FOUNDATION COMPONENTS.*FOR MORE INFORMATION, SEE TECHNICAL

7. PARTIAL HEIGHT BREAKAWAY WALL SYSTEMS ARE NOT

| 10d | 8d | 10d | 8d | 10d | | |
|-----|----|-----|-------|-----|--|--|
| 28 | 47 | 30 | 50 | 32 | | |
| | | | | | | |
| | | | | | | |
| | 1 | 5 | 16 | | | |
| 10d | 8d | 10d | 8d | 10d | | |
| 38 | 57 | 40 | 61 43 | | | |
| | | | | | | |
| | | | | | | |
| | 1 | 5 | 16 | | | |

10d

47

8d

75

10d

50

16

15

10d

44

8d

70

PROJECT #: DATE:

SCALE:

N.T.S.

RESPONSIBILITIES:

- 1. GENERAL CONTRACTOR IS RESPONSIBLE FOR VERIFYING STRUCTURAL DRAWINGS & ARCHITECTURAL DRAWINGS ARE IN AGREEMENT. ANY DISCOURSE BETWEEN THE TWO SETS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER & ARCHITECT FOR RESOLUTION.
- 2. GENERAL CONTRACTOR IS RESPONSIBLE FOR VERIFYING STRUCTURAL DRAWINGS TO CONFIRM THE AVAILABILITY OF ALL REQUIRED DETAILS. IF ANY REQUIRED INFORMATION IS NOT LISTED IN THE STRUCTURAL DRAWINGS, GENERAL CONTRACTOR IS REQUIRED TO CONTACT ENGINEER OF RECORD FOR PARTICULAR INFORMATION.
- 3. GENERAL CONTRACTOR IS RESPONSIBLE FOR ENSURING CONSTRUCTION ADHERES TO FEMA FLOOD PLAIN, LOCAL MUNICIPALITY, (FFE) FINISH FLOOR ELEVATION REQUIREMENTS AND/OR DISASTER RECOVERY CONSTRUCTION GUIDELINES.
- 4. GENERAL CONTRACTOR IS RESPONSIBLE FOR COMPLIANCE WITH ALL LOCAL MUNICIPALITY CODE REQUIREMENTS AND/OR DISASTER RECOVERY CONSTRUCTION GUIDELINES
- COBALT ENGINEERING IS NOT RESPONSIBLE FOR DESIGN FLOOD ELEVATION OR ANY REQUIREMENT EXCEEDING THE BASE FLOOD ELEVATION AS SHOWN ON THE PROVIDED ELEVATION CERTIFICATE.
- 6. COBALT ENGINEERING & INSPECTIONS LLC. IS NOT RESPONSIBLE FOR THE PERFORMANCE OF FOUNDATION AS A RESULT OF THE BEHAVIOR OF THE SUPPORTING SOIL AND/OR DIFFERENTIAL SETTLEMENT DUE TO SEASONAL CHANGES SUCH AS DROUGHT. EXTENSIVE RAIN AND OTHER DRASTIC CLIMATE CHANGES.

CONCRETE:

- 1. UNLESS OTHERWISE NOTED, ALL CONCRETE FOUNDATION WALLS AND SLABS ON GRADE SHALL BE 3,000 PSI (28 DAY COMPRESSION STRENGTH) CONCRETE, PLACE CONCRETE SLABS ON 4" OF COMPACTED LOW P.I SAND FILL. ALL SLABS UNDER INTERIOR FINISHED AND HEATED LIVING SPACES SHALL BE PLACED ON 6 MIL POLYETHYLENE VAPOR BARRIER WITH A MINIMUM OF 6" LAPPED JOINTS. ALL REBAR LAP SPACING LENGTHS SHALL BE MINIMUM 50 TIMES THE BAR SIZE.
- 2. PROVIDE ¹/₂" EXPANSION JOINT MATERIAL BETWEEN ALL CONCRETE SLABS ABUTTING CONCRETE OR MASONRY WALLS OCCURRING IN EXTERIOR OR UNHEATED SPACES OR AREAS.
- 3. CONCRETE FOR ALL BASEMENT WALLS, FOUNDATION WALLS, PORCHES, WALKS, PATIOS, STEPS, GARAGE, CARPORT FLOOR SLABS AND DRIVEWAYS SHALL BE AIR-ENTRAINED.
- 4. ALL REINFORCEMENT STEEL SHALL MEET ASTM A615 GRADE 60 SPECIFICATIONS.
- 5. REBAR COVER: PROVIDE THE FOLLOWING CONCRETE PROTECTIVE COVERINGS FOR REINFORCEMENT, UNLESS NOTED OTHERWISE.
- A. 3" FOR ALL CONCRETE DEPOSITED DIRECTLY AGAINST THE GROUND. B. 2" FOR ALL FORMED CONCRETE EXPOSED TO WEATHER OR IN CONTACT WITH THE
- GROUND. (UNLESS NOTED OTHERWISE)
- 6. DEVELOPMENTAL LENGTH (DL) SHALL BE 50 TIMES THE DIAMETER OF THE REBAR

RESPONSIBILITIES:

A. ALL (EARTHWORK, EXCAVATIONS, STRUCTURAL FILL, COMPACTION, ETC) SHALL BE DONE IN ACCORDANCE WITH GEO-TECHNICAL REPORT

1. EXCAVATING

- A ALL EXCAVATION SHALL COMPLY WITH OSHA STANDARDS
- B. DEMOLISH AND REMOVE ALL OBSTRUCTIONS AS REQUIRED. AREAS TO RECEIVE FOUNDATION SHALL BE STRIPPED TO REMOVE ALL ORGANIC MATERIAL AND CONTAMINATED OR SOFT SOIL. DISPOSAL OF ALL DEBRIS FROM DEMOLITION AND STRIPPING OPERATIONS SHALL BE AS SPECIFIED BY OWNER.
- C. CARE SHALL BE TAKEN TO NOT OVER EXCAVATE BELOW BOTTOM OF FOUNDATION. ANY OVER EXCAVATION REQUIRED FOR REMOVAL OF THE EXISTING FILL SHALL BE BACKFILLED PER SECTION 2.A OR 2.B
- D. ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY PROOF-ROLLING PER TX DOT ITEM 216 TO DETERMINE THAT ALL LOOSE, SOFT, OR OTHERWISE UNDESIRABLE MATERIALS ARE REMOVED. IF AN AREA OF UNDESIRABLE MATERIAL IS DISCOVERED AT THE BOTTOM OF THE EXCAVATION, IT SHALL BE REMOVED AND REPLACED WITH COMPACTED BACKFILL PER SECTION 2.A. OR 2.B.
- E. THE UPPER 6" OF EXPOSED SOILS SHALL BE COMPACTED TO AT LEAST 95% OF MAX. DRY DENSITY DETERMINED BY MODIFIED PROCTOR TEST (ASTM D1557)
- F. WHERE SOIL CONDITIONS PERMIT, FOUNDATIONS BELOW GRADE MAY BE EARTH FORMED UNLESS OTHERWISE NOTED.

2. ENGINEERED FILL (CLAY)

- A. CLAY STRUCTURE FILL SHALL BE SANDY CLAY WITH LIQUID LIMIT OF LESS THAN 35 AND PLASTICITY INDEX (PI) BETWEEN 8 AND 20.
- B. CLAY SHALL BE MOISTURE CONDITIONED WITHIN 2% OF OPTIMUM MOISTURE CONTENT AND COMPACTED TO AT LEAST 95 % OF THE MAX DRY DENSITY DETERMINED BY THE MODIFIED PROCTOR TEST (ASTM D1557) WITH 8" MAX. LOOSE LIFTS.
- C. FOR COMPACTION BY MANUALLY-GUIDED POWER COMPACTORS, STRUCTURAL FILL SHALL BE PLACED IN LIFTS OF 6" MAXIMUM LOOSE THICKNESS.

NOTES ON PRESSURE TREATED LUMBER:

GALVANIZED

FASTENERS (AND OTHER METAL PRODUCTS) FOR USE WITH WOOD TREATED WITH ACQ PRESERVATIVES INCLUDE: HOT-DIP GALVANIZED (THE MINIMUM STANDARD) THE MINIMUM HOT-DIP GALVANIZED REQUIREMENT FOR USE WITH TREATED WOOD SHOULD CONFORM TO THE FOLLOWING ASTM STANDARDS: ASTM- A153 (FOR HOT-DIP FASTENER PRODUCTS) AND ASTM-A653 (COATING DESIGNATION g-185 FOR HOT-DIP CONNECTOR AND SHEET PRODUCTS).

STAINLESS STEEL STAINLESS STEEL FASTENERS AND CONNECTORS ARE REQUIRED FOR PERMANENT WOOD FOUNDATIONS BELOW GRADE AND ARE RECOMMENDED FOR USE WITH TREATED WOOD IN OTHER SEVERE EXTERIOR APPLICATIONS SUCH AS SWIMMING POOLS, SALT WATER EXPOSURE, ETC. - TYPE 304 AND 316 ARE THE RECOMMENDED GRADES TO US

ALUMINUM SHOULD NOT BE USED IN DIRECT CONTACT WITH PRODUCTS TREATED WITH ACQ PRESERVATIVE SPACER MATERIALS OR OTHER PHYSICAL BARRIERS ARE RECOMMENDED TO PREVENT DIRECT CONTACT OF ACQ TREATED WOOD WITH ALUMINUM PRODUCTS.

STRUCTURAL:

IF TRUSSES ARE SPECIFIED ON THE PLANS. THE TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND/OR STRESS AND LOAD CALCULATIONS (DIAGRAMS) FOR CONTRACTORS APPROVAL PRIOR TO CONSTRUCTION. DRAWINGS SHALL BEAR SEAL OF THE REGISTERED ENGINEER IN THE STATE IN WHICH THE STRUCTURE IS BUILT.

MISC. BOLTS AND THREADED FASTENERS

- A. SPECIFICATION
- B. DESIGN
- C. INSTALLATION

1603.1.4 WIND DESIGN DATA:

THE FOLLOWING INFORMATION RELATED TO WIND LOADS SHALL BE SHOWN, REGARDLESS OF WHETHER WIND LOADS GOVERN THE DESIGN OF THE LATERAL FORCE-RESISTING SYSTEM OF THE STRUCTURE:

- RISK CATEGORY. 2.
- UTILIZED

ALL WOOD MEMBERS (INCLUDING PLYWOOD SHEATHING & ALL WOOD BASED MATERIALS) IN CONTACT WITH CONCRETE. OR EXPOSED TO WEATHER. MOISTURE OR WITHIN 18" OF THE GROUND (SUCH AS PORCH & BALCONY FRAMING) SHALL BE PRESSURE- TREATED.

CURRENTLY, THE PRODUCT COMMONLY USED FOR PRESSURE TREATMENT IS ALKALINE COPPER QUATERNARY (ACQ). THIS MATERIAL IS EXTREMELY CORROSIVE. ONLY HOT- DIPPED

ANCHOR BOLTS, THRU BOLTS, NAILS, OR OTHER CORROSIVE-RESISTANT FASTENERS, SHALL BE USED WITH ACQ-TREATED LUMBER. FASTENER MANUFACTURER OR SUPPLIER SHALL BE CONSULTED ON THE SUITABILITY OF GALVANIZED FASTENERS FOR USE WITH TREATED LUMBER.

1. BOLTS SHALL CONFIRM TO ASTM A307 GRADE A. CARBON STEEL EXTERNALLY THREADEDFASTENERS, U.N.O.

2. BOLTS AND NUTS SHALL BE HEX HEAD ASTM A307 AND CONFORM TO ANSI STANDARDS B18.2.1 AND B18.2.1 AS WELL AS ASTM MATERIAL STANDARDS ASTM 307. 3. WASHERS SHALL BE CIRCULAR, FLAT AND SMOOTH IN CONFORMANCE WITH THE REQUIREMENTS OF TYPE A WASHERS IN ANSI STANDARD B23.1.

1. THE MINIMUM BOLT DIAMETER SHALL BE $\frac{1}{2}$ INCH AND BE BEARING TYPE CONNECTION USING STANDARD HOLES WITH THREADS EXCLUDED FROM THE SHEAR PLANE, U.N.O.

1. A307 BOLTS SHALL BE TIGHTENED PER TURN-OF-NUT BOLTING METHOD. THE TURN-OF-NUT BOLTING METHOD CONDITION DEFINED AS THE TIGHTNESS ATTAINED BY THE EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH, THEN BACKED OFF 2/3 TURN. (PER AISC MANUEL OF STEEL CONSTRUCTION, LOAD & RESISTANCE FACTOR 3RD EDITION 8.2.1) DO NOT OVERTIGHTEN BOLTS AS TO DAMAGE THE WOOD ELEMENTS.

2. BOLTS AND NUTS SHALL BE WELL LUBRICATED AT TIME OF INSTALLATION, DRY, CORRODED BOLTS WILL NOT BE ALLOWED.

3. ALL BOLTS SHALL BE NEW AND NOT REUSED.

4. BOLTED CONNECTIONS SHALL BE KNURLED OR SPOT-WELDED TO PREVENT BACK-OUT.

BASIC DESIGN WIND SPEED, V, MILES PER HOUR AND ALLOWABLE STRESS DESIGN WIND SPEED, VASD, AS DETERMINED IN ACCORDANCE WITH SECTION 1609.3.1.

3. WIND EXPOSURE. APPLICABLE WIND DIRECTION IF MORE THAN ONE WIND EXPOSURE IS

4. APPLICABLE INTERNAL PRESSURE COEFFICIENT.

DESIGN WIND PRESSURES TO BE USED FOR EXTERIOR COMPONENT AND CLADDING MATERIALS NOT SPECIFICALLY DESIGNED BY THE REGISTERED DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN OF THE STRUCTURE, PSF (KN/M2).

THIS STRUCTURE IS IN A WIND COASTAL MEAN HIGH WATER LI GREATER WIND ZONE. OR

IN AN AREA WHERE THE ULTIMA

WIND BORNE DEBRIS PROTECT OPENING BEING COVERED.

CODES & DESIGN LOADS

CODE: FLORIDA BUILDING CODE- 2020

ZONE LEGEND

(MIN.) DESIGN PRESSURE FOR

ALL AREAS NOT DESIGNATED A ZONE 5 AS INDICATED ARE ZON

WINDOWS, DOORS AND WALLS THE ALPHA VALUE DEFINES THE SIZE OF ZONE 5 (MEASURED FR ANY OUTSIDE CORNER)

SHOULD BE 10% OF α = 3'-0" < VERIFY WITH PLANS

| | | COMPONE | N |
|------|---------|---------|---|
| ZONE | DOOR | WINDOW | 1 |
| 1 | | | |
| 2 | | | |
| 3 | | | |
| 4 | +32/-34 | +34/-36 | |
| 5 | +32/-41 | +34/-45 | |
| ALL | | | |

EXPOSURE = C

INTERNAL PRESSURE COEFFIC PRESSURE AREA 10 SQ. FT.

DESIGN LOADS:

1. ROOF LIVE LOADS 20 PSF (2. FLOOR LIVE LOADS

| USE | | LO | | | |
|------------------------|-----------------------|------|--|--|--|
| EXT | ERIOR BALCONIES | | | | |
| DEC | KS | | | | |
| FIRE | E ESCAPES | | | | |
| STA | IRS/RAMPS | | | | |
| GUARDRAILS & HANDRAILS | | | | | |
| (a) | ELEVATED GARAGE FLC | | | | |
| (b) | NO STORAGE LOAD IS F | REQ | | | |
| (c) | INDIVIDUAL STAIR TREA | DS | | | |
| | UNIFORMLY DISTRIBUTE | ED I | | | |
| | CONCENTRATED LOAD | AC | | | |
| | INCHES, WHICHEVER PR | ROE | | | |

(d) A SINGLE CONCENTRATE

POINT ALONG THE TOP.

3. WIND LOADS (AMERICAN SO BASIC WIND DESIGN VELOCITY

4. DEAD LOAD

| USE | LOAD (|
|-------|--------|
| ROOF | 10 |
| DECKS | 10 |
| FLOOR | 10 |
| WALLS | 11 |
| | • |

| | WALL TYPE |
|---------|--------------|
| 1 STORY | EXTERIOR WA |
| HOUSE | INTERIOR WAI |
| | PLUMBING WA |
| | |

NOTES:

1. WALL SHEATHING SHALL BE EXTERIOR COVERINGS SHALL

DIMENSIONS: 1. DRAWING DIMENSIONS GOV

SELECTED DOORS, WINDOW BEGINS.

| FASTENER CORRO | ç |
|-------------------------|---|
| MOISTURE EXPOSURE | |
| EXTERIOR | |
| ENCLOSED BUT VENTILATED | |
| AIR CONDITIONED SPACE | |

FLORID

| | DRAWING LIST |
|--|---|
| ORIDA WIND BORNE DEBRIS PROTECTION | TABLE OF CONTENTS XX-XXXX-GN-1.00 GENERAL NOTES 1 |
| ND BORNE DEBRIS PROTECTION AREAS. IT IS WITHIN 1 MILE OF THE R LINE WHERE THE ULTIMATE WIND SPEED, Vult. AND IS 130MPH OR | XX-XXX-GN-2.00GENERAL NOTES 2XX-XXX-F-1.00FOUNDATION & CMU WALL PLANSXX-XXXX-F-2.00FOUNDATION DETAILS |
| IMATE DESIGN WIND SPEED, Vult. IS 140MPH OR GREATER. | XX-XXXX-S-1.00CEILING JOIST & RAFTER PLANSXX-XXXX-SD-1.00STANDARD DETAILS 1 |
| ECTIONS SHALL MATCH OR EXCEED THE DESIGN PRESSURE FOR THE | XX-XXXX-SD-2.00 STANDARD DETAILS 2 |
| | |
| 020 (CITY OF FORT PIERCE REQUIREMENTS) | |
| DR WINDSTORM COMPLIANCE) | |
| ZONE 4 | |
| LS THE | TYPICAL DETAIL OR SECTION CALLOUT |
| FROM | DETAIL PAGE |
| OF SHORTEST WALL NS | (XX/XX-X.XX) |
| | ABBREVIATIONS |
| +19/-30 | APB. ANTHONY POWER BEAM APP. ANTHONY POWER PRESERVED |
| +19/-53 +19/-79 | CANT. CANTILEVER CONT. CONTINUOUS |
| | DF. DOUGLAS FIK DWG. DRAWING FND FOLINDATION |
| +25/-27 +28/-34 | F.V. FIELD VERIFY HDR. HEADER |
| FICIENT = 0.18 | PLCS. PLACES SIM. SIMILAR |
| | S.F. STEEL FLITCH SPF. SPRUCE PINE FIR |
| SF (SUBJECT TO SLOPE & TRIBUTARY AREA REDUCTION FACTORS) | SYP. SOUTHERN YELLOW PINE TRPL. TRIPLE |
| LOAD (PSF) USE LOAD (PSF) | |
| 40SLEEPING ROOMS3040OTHER ROOMS40 | |
| 40 ATTIC W/STORAGE 20 (b) 40 (c) ATTIC W/O STORAGE 10 (b) | 1. ALL SPECIFICATIONS ARE MINIMUM REQUIREMENTS. SPECIFICATIONS MAY BE |
| 250(d) GARAGE N/A(a) | INCREASED TO MEET ARCHITECTURAL OR CONSTRUCTION PREFERENCES. |
| PLIED OVER A 20-SQUARE-INCH AREA. REQUIRED WITH ROOF SLOPES OF 3 IN 12, OR | |
| ADS SHALL BE DESIGNED FOR THE | Revisions: # DATE DATE DESCRIPTION OF CHANGE |
| ED LIVE LOAD OR A 300-POUNDS ACTING OVER AN AREA OF 4 SQUARE | |
| TED LOAD APPLIED IN ANY DIRECTION @ ANY | |
| SOCIETY OF CIVIL ENGINEERS, 7-16) TY 160 MPH (ULTIMATE WIND SPEED) RISK CATEGORY: II | ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR UTILIZED IN ANY FORM WITHOUT PRIOR WRITTEN AUTHORIZATION OF "COBALT". |
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| 40 (PSF) 10 | SHALL IT BE USED FOR PERMITTING PURPOSES. SITE |
| <u>10</u> <u>10</u> | BASIS AND ANY USE BEYOND THE AFOREMENTIONED INTENT IS PROHIBITED. |
| 11 MINIMUM WALL FRAMING REQUIREMENTS | |
| YPE MATERIAL & SPACING ANCHOR BOLT REQUIREMENTS | - |
| X WALLS 2X6 SYP #2 @ 16" O.C. %8"X10" J-BOLTS W/ 3" SQ. X .125"THK. WASHER @ 32" O.C. | - |
| VVALLS ZA4 STP #Z @ 10 0.0. WALLS 2X6 SYP #2 @ 16" O.C. | - |
| BE AS INDICATED ON SHEARWALL & HOLDOWN PLAN. LL CONFORM TO FBC INSTALLATION INSTRUCTIONS. | |
| OVERN OVER SCALE. VERIFY ALL ROUGH OPENING DIMENSIONS FOR OWS AND MECHANICAL REQUIREMENTS BEFORE CONSTRUCTION | |
| ROSION RESISTANCE | |
| FINISH | |
| HOT DIPPED GALV. (MIN.) | |
| ELECTRO GALVANIZED (MIN.) | |
| EPOXY COATED (MIN.) | DRAWN BY: AW CHECKED BY: CCH |
| | PROJECT #:SCALE:N.T.S.DATE:GN-1.00 |

| NAILING | SCHEDULE | | |
|---|---------------------------|------------------------|---------------------|
| JOINT DESCRIPTION | NUMBER OF COMMON NAILS | NUMBER OF BOX NAILS | NAIL SPACING |
| WALL FRAM | MING | | |
| Top plate to top plate (face-nailed) | 2-16d | 2-16d | per foot |
| Top plates at intersections (face-nailed) | 4-16d | 5-16d | joints - each side |
| Stud to stud (face-nailed) | 2-16d | 2-16d | 24"o.c. |
| Header to header (face-nailed) | 16d | 16d | 16"o.c. along edges |
| Top or bottom plate to stud (end-nailed) | 1 -16d | 1 -40d | per stud |
| Bottom plate to floor joist, bandjoist, endjoist or blocking (face-nail | ed) 2-16d | 2-16d | per foot |
| FLOOR FRA | MING | | |
| Joist to sill, top plate or girder (toe-nailed) | 4-8d | 4-10d | per joist |
| Bridging to joist (toe-nailed) | 2-8d | 2-10d | each end |
| Blocking to joist (toe-nailed) | 2-8d | 2-10d | each end |
| Blocking to sill or top plate (toe-nailed) | 3-16d | 4-16d | each block |
| Ledger strip to beam (face-nailed) | 3-16d | 4-16d | each joist |
| Joist on ledger to beam (toe-nailed) | 3-8d | 3-10d | per joist |
| Band joist to joist (end-nailed) | 3-16d | 4-16d | per joist |
| Band joist to sill or top plate (toe-nailed) | 2-16d | 3-16d | per foot |
| CEILING SHE | ATHING | | |
| Gypsum wallboard | 5d coolers | 5d coolers | 7"edge/10"field |
| ROOF FRA | MING | | |
| Rafter to top plate (toe-nailed) | 5 -8d | 5 -10d | per rafter |
| Ceiling joist to top plate (toe-nailed) | 5 -8d | 5 -10d | per joist |
| Ceiling joist to parallel rafter (face-nailed) | 7 -16d | 7 -40d | each lap |
| Ceiling joist laps over partitions (face-nailed) | 7 -16d | 7 -40d | each lap |
| Collar tie to rafter (face-nailed) | 3 -8d | 3 -10d | per tie |
| Blocking to rafter (toe-nailed) | 2-8d | 2 -10d | each end |
| Rim board to rafter (end-nailed) | 2-16d | 3 -16d | each end |

| r. | | | | | | | |
|----|--|---|--|------------------|--|--|--|
| | CONCRETE COVERAGE FOR CAST-IN-PLACE (NON PRE-STRESSED CONCRETE MEMBERS) | | | | | | |
| | CONCRETE EXPOSURE | MEMBER | MEMBER REINFORCEMENT | | | | |
| | CAST AGAINST AND PERMANENTLY IN CONTACT W/ GROUND | ALL | ALL | 3" | | | |
| EX | EXPOSED TO WEATHER | ALL | NO. 6 THROUGH NO. 18 BARS | 2" | | | |
| | WITH GROUND | | NO. 5, W31 / D31 WIRE & SMALLER | 1-1⁄2" | | | |
| | | SLABS, | NO. 14 & NO. 18 BARS | 1-1⁄2" | | | |
| | | WALLS | NO. 11 BARS & SMALLER | ³ ⁄4" | | | |
| | CONTACT WITH GROUND | BEAMS, COLUMNS, PEDESTALS, & TENSION TIES | PRIMARY REINFORCEMENT, STIRRUPS, TIES, SPIRALS, & HOOPS | 1-1⁄2" | | | |

| | DEVELOPMENT LENGTH CHART | | | | | | |
|-------------|--------------------------|-------------------------------|-----|--------|-------|--|--|
| | SLAB/ | WA | LLS | | | | |
| BAR SIZE | 12" THICKNESS OR LESS | THICKNESS GREATER THAN 12" | | HORIZ. | VERT. | | |
| | ALL BARS | BOTTOM OTHER BARS BARS | | | | | |
| #3 | 17" | 17" | 22" | 17" | 22" | | |
| #4 | 22" | 22" | 30" | 22" | 30" | | |
| #5 | 29" | 29" | 36" | 29" | 36" | | |

CONSTRUCTIV

| R | evisions: | I | | |
|-----------------|---------------------------------|---------------------------------------|---|----------------------------------|
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| ALL F UTILIZ | RIGHTS RESERV ZED IN ANY FOF | ED. NO PART OF THI M WITHOUT PRIOR | S DOCUMENT MAY BE R WRITTEN AUTHORIZATIO | REPRODUCED OR ON OF "COBALT". |
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| | HIS DRAW | ING IS A CONC | EPTUAL PLAN AN | D IS NOT |
| | IENDED I (HALL IT RE | J BE USED AS A | A SITE SPECIFIC I | PLAN NOR SES SITE |
| SF | PECIFICS S | HALL BE PREP | ARED ON A CASE | -BY-CASE |
| BA | ASIS AND A | NY USE BEYON | ND THE AFOREME | ENTIONED |
| | | INTENT IS P | ROHIBITED. | |
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| PROJE | ECT #: | | SCALE: | N.T.S. |
| DATE: | | | | GN-2.00 |

FOUNDATION PLAN (STANDARD) 〔01〕

ALL OPTIONS

CMU WALL PLAN (STANDARD) (02)

ALL OPTIONS

LEGEND

| R # | evisions: DATE | DESCRIPTION OF CHANGE |
|-------------------------------|---|---|
| UTILIZ T IN SF B/ | THIS DRAW TENDED TO ALL IT BE PECIFICS S ASIS AND A | ING IS A CONCEPTUAL PLAN AND IS NOT D BE USED AS A SITE SPECIFIC PLAN NOR USED FOR PERMITTING PURPOSES. SITE HALL BE PREPARED ON A CASE-BY-CASE NY USE BEYOND THE AFOREMENTIONED INTENT IS PROHIBITED. |
| | | |
| | | |

CEILING JOIST PLAN (STANDARD) 01

ALL OPTIONS

| | | | LEGEND | |
|----------|--|--|--|--|
| | LUS26 OR LUC26Z CONCEALED | | | G |
| (2) 2X6 | LUS26-2 OR HUC26-2 CONCEALED | | TYPICAL STRONG BAC | - K SEE |
| 2X8 | LUS28 OR LUC28Z CONCEALED | | DETL. #4 ON DWG. SD-2 | 2.00 |
| (2) 2X8 | LUS28-2 OR HUC28-2 CONCEALED | | PONY WALL TO ROOF [| DIAPHRAGM |
| 2X10 | LUS210 OR LUC210Z CONCEALED | • | BRACE POINT | |
| (2) 2X10 | HUS210-2 OR HUC210-2 CONCEALED | | BW - BRACE TO WALL | |
| 2X12 | HUS210 OR LUC210Z CONCEALED | | BJ - BRACE TO JOIST | |
| SOME MO | DELS MAY NOT BE USED ING PLAN FOR LOCATION) | | BS - BRACE TO STRON | GBACK |
| | | CEILING FRAM 1. ALL CEILIN O.C. (U.N.C 2. ATTIC ACC 350LB CAP HEADER FRA 1. REFER TO HEADER FRAMIN 1. REFER TO HEADER R SPECIFIED STUD PACK F 1. (4) 2X STU SOLID SAV ROOF FRAMIN 1. RIDGES TO 2. ALL HIPS T 3. ALL RAFTE 4. ROOF SHE PLYWOOD AROUND P EDGE & 6" SECTION F 5. ROOF UND R905.1.1.1 FRAMING: 1. ALL STUDS ETC. SHAL DIRECTLY | NOTES AING: G JOISTS SHALL BE 2X6 = 0.) ESS SHALL BE NO LESS ACITY FOLDING STAIR SY AMING: DETAIL 1 ON SD-1.00 FO EQUIREMENTS. (UNLESS ON PLAN) FRAMING: D PACK UNDER EACH EN VN BEAM. NG: D BE 2x10 #2 SYP TO BE 2x8 | #2 SYP 16" THAN YSTEM. R ALL O OF 2X O.C. (UNO) K NAILS, 4" M. 6" R IPLY WITH RS, TRUSS, D BEAR |
| | | Revisions: | | |
| | | # DATE | DESCRIPTION OF CHA | NGE |
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| | | ALL RIGHTS RESERVED. I UTILIZED IN ANY FORM W | NO PART OF THIS DOCUMENT MAY BE /ITHOUT PRIOR WRITTEN AUTHORIZAT | REPRODUCED OR TON OF "COBALT". |
| | | THIS DRAWING INTENDED TO BE SHALL IT BE USE SPECIFICS SHAI BASIS AND ANY I | IS A CONCEPTUAL PLAN AN E USED AS A SITE SPECIFIC ED FOR PERMITTING PURPO L BE PREPARED ON A CAS USE BEYOND THE AFOREM NTENT IS PROHIBITED. | ND IS NOT PLAN NOR DSES. SITE E-BY-CASE ENTIONED |
| V | | | | |
| | | | | 1/1 4 01 |

| KING AND JACK STUD SCHEDULE | | | |
|--|---|-------------------|--------------------------------------|
| OPENING SPAN | KING/JACK STUDS REQUIRED PER OPENING SPAN | | |
| 4' OR LESS | (1) KING & (1) JACK STUD (EACH SIDE) | | |
| 4' TO 6' OPENING(2) KING & (2) JACK STUD (EACH SIDE)6' TO 8' OPENING(3) KING & (2) JACK STUD (EACH SIDE) | | | |
| | | 8' TO 12' OPENING | (4) KING & (3) JACK STUD (EACH SIDE) |
| 12' TO 16' OPENING | (5) KING & (4) JACK STUD (EACH SIDE) | | |

SINGLE STORY HEADER SCHEDULE

| ALL H | EADERS ARE DROP BEA | MS, U.N.O | GRADE # 2 LUMBER |
|-------|---------------------|-----------------|--|
| TYPE | OPENING SPAN | WALL THICKNE | HEADER REQ'D SS |
| 1 | 3' OR LESS | 3.5" | TWO 2"x8" #2 SYP w/ 1/2" PLYWD FLITCH |
| 2 | 3' OR LESS | 5.5" | THREE 2"x8" #2 SYP w/ 1/2" PLYWD FLITCHS |
| 3 | 3' TO 5' | 3.5" | TWO 2"x10" # 2 SYP w/ 1/2" PLYWD FLITCH |
| 4 | 3' TO 5' | 5.5" | THREE 2"x10" #2SYP w/ 1/2" PLYWD FLITCHS |
| 5 | 5' TO 8' | 3.5" | TWO 2"x12" #2 SYP w/ 1/2" PLYWD FLITCH |
| 6 | 5' TO 8' | 5.5" | THREE 2"x12" #2 SYP w/ 1/2" PLYWD FLITCHS |
| | | | |

OPENINGS GREATER THAN 6'-1" SHALL BE AS INDICATED ON PLAN DRAWINGS NOTES:

THIS TABLE USES EITHER 0.25" DEFLECTION OR L/240 WHICHEVER IS LESS 1

THIS TABLE ACCOUNTS FOR ONLY ROOF AND CEILING LOADING 2.

FOR GENERIC 1 STORY FRAMING 3. FOR 2-STORY DWELLINGS USE 2X12 HEADERS ON FIRST FLOOR LIVING SPACE (UNO) 4.

ALL NON-LOAD BEARING HEADERS SHALL BE 2X6 #2 SYP W/ $\frac{1}{2}$ " PLYWOOD FLITCH. *

INTEREIOR HEADER SCHEDULES 1

(3) BEAM TO FILLED CMU WALL DETAIL

SCHEDULE

CLIP & STRAP SCHEDULE

| CONNECTION LOCATION | CLIP / STRAP |
|--------------------------------|----------------------|
| STUD TO BOTTOM PLATE (SLAB) | SIMPSON H2.5A 1 |
| STUD TO STRINGER/BEAM (PILING) | SIMPSON LSTA36 23 |
| STUD TO STUD BETWEEN LEVELS | SIMPSON LSTA36 23 |
| HEADER STRAPS (8' PLATE) | SIMPSON LSTA36 13 |
| HEADER STRAPS (9' PLATE) | SIMPSON CS16 (48") 1 |
| 2X RAFTER TO TOP PLATE/ BEAM | SIMPSON H2.5A 1 |
| RAFTER TO RAFTER @ RIDGE | SIMPSON LSTA18 1 |
| RAFTER TO RAFTER @ HIP | SIMPSON LSTA18 1 |
| ROOF TRUSS TO STUD | SIMPSON HTS30C 1 |
| RAFTER TO TOP PLATE | (2) SIMPSON H2.5A 1 |
| STUD TO TOP PLATE | (2) SIMPSON H2.5A 1 |
| | |

1. FILL ALL NAIL HOLES U.N.O.

- 2. STUD STRAPS SHALL TERMINATE AT A BEAM OR STRINGER (NEVER AT BAND JOIST) & EXTEND PAST RIM JOIST BY A MINIMUM OF 10".
- 3. SIMPSON CS16 (OF EQUAL LENGTH AND FASTENERS) MAY BE SUBSTITUTED FOR SIMPSON LSTA
- 4. FASTENERS & QUANTITY PER MANUFACTURER'S
- INSTALLATION INSTRUCTIONS.
- 5. SOME CLIPS & STRAPS LISTED MAY NOT APPLY TO EVERY PROJECT.

| # | DATE | DESCRIPTION OF CHANGE | | |
|--|--------------------------------|--|--|--|
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| ALL F UTILIZ | RIGHTS RESERV ED IN ANY FOR | ED. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR M WITHOUT PRIOR WRITTEN AUTHORIZATION OF "COBALT". | | |
| THIS DRAWING IS A CONCEPTUAL PLAN AND IS NOT INTENDED TO BE USED AS A SITE SPECIFIC PLAN NOR SHALL IT BE USED FOR PERMITTING PURPOSES. SITE SPECIFICS SHALL BE PREPARED ON A CASE-BY-CASE BASIS AND ANY USE BEYOND THE AFOREMENTIONED INTENT IS PROHIBITED. | | | | |

RESPONSIBILITIES:

- 1. GENERAL CONTRACTOR IS RESPONSIBLE FOR VERIFYING STRUCTURAL DRAWINGS & ARCHITECTURAL DRAWINGS ARE IN AGREEMENT. ANY DISCOURSE BETWEEN THE TWO SETS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER & ARCHITECT FOR RESOLUTION.
- 2. GENERAL CONTRACTOR IS RESPONSIBLE FOR VERIFYING STRUCTURAL DRAWINGS TO CONFIRM THE AVAILABILITY OF ALL REQUIRED DETAILS. IF ANY REQUIRED INFORMATION IS NOT LISTED IN THE STRUCTURAL DRAWINGS, GENERAL CONTRACTOR IS REQUIRED TO CONTACT ENGINEER OF RECORD FOR PARTICULAR INFORMATION.
- 3. GENERAL CONTRACTOR IS RESPONSIBLE FOR ENSURING CONSTRUCTION ADHERES TO FEMA FLOOD PLAIN, LOCAL MUNICIPALITY, (FFE) FINISH FLOOR ELEVATION REQUIREMENTS AND/OR DISASTER RECOVERY CONSTRUCTION GUIDELINES.
- 4. GENERAL CONTRACTOR IS RESPONSIBLE FOR COMPLIANCE WITH ALL LOCAL MUNICIPALITY CODE REQUIREMENTS AND/OR DISASTER RECOVERY CONSTRUCTION GUIDELINES
- COBALT ENGINEERING IS NOT RESPONSIBLE FOR DESIGN FLOOD ELEVATION OR ANY REQUIREMENT EXCEEDING THE BASE FLOOD ELEVATION AS SHOWN ON THE PROVIDED ELEVATION CERTIFICATE.
- 6. COBALT ENGINEERING & INSPECTIONS LLC. IS NOT RESPONSIBLE FOR THE PERFORMANCE OF FOUNDATION AS A RESULT OF THE BEHAVIOR OF THE SUPPORTING SOIL AND/OR DIFFERENTIAL SETTLEMENT DUE TO SEASONAL CHANGES SUCH AS DROUGHT. EXTENSIVE RAIN AND OTHER DRASTIC CLIMATE CHANGES.

CONCRETE:

- 1. UNLESS OTHERWISE NOTED, ALL CONCRETE FOUNDATION WALLS AND SLABS ON GRADE SHALL BE 3,000 PSI (28 DAY COMPRESSION STRENGTH) CONCRETE, PLACE CONCRETE SLABS ON 4" OF COMPACTED LOW P.I SAND FILL. ALL SLABS UNDER INTERIOR FINISHED AND HEATED LIVING SPACES SHALL BE PLACED ON 6 MIL POLYETHYLENE VAPOR BARRIER WITH A MINIMUM OF 6" LAPPED JOINTS. ALL REBAR LAP SPACING LENGTHS SHALL BE MINIMUM 50 TIMES THE BAR SIZE.
- 2. PROVIDE ¹/₂" EXPANSION JOINT MATERIAL BETWEEN ALL CONCRETE SLABS ABUTTING CONCRETE OR MASONRY WALLS OCCURRING IN EXTERIOR OR UNHEATED SPACES OR AREAS.
- 3. CONCRETE FOR ALL BASEMENT WALLS, FOUNDATION WALLS, PORCHES, WALKS, PATIOS, STEPS, GARAGE, CARPORT FLOOR SLABS AND DRIVEWAYS SHALL BE AIR-ENTRAINED.
- 4. ALL REINFORCEMENT STEEL SHALL MEET ASTM A615 GRADE 60 SPECIFICATIONS.
- 5. REBAR COVER: PROVIDE THE FOLLOWING CONCRETE PROTECTIVE COVERINGS FOR REINFORCEMENT, UNLESS NOTED OTHERWISE.
- A. 3" FOR ALL CONCRETE DEPOSITED DIRECTLY AGAINST THE GROUND. B. 2" FOR ALL FORMED CONCRETE EXPOSED TO WEATHER OR IN CONTACT WITH THE
- GROUND. (UNLESS NOTED OTHERWISE)
- 6. DEVELOPMENTAL LENGTH (DL) SHALL BE 50 TIMES THE DIAMETER OF THE REBAR

RESPONSIBILITIES:

A. ALL (EARTHWORK, EXCAVATIONS, STRUCTURAL FILL, COMPACTION, ETC) SHALL BE DONE IN ACCORDANCE WITH GEO-TECHNICAL REPORT

1. EXCAVATING

- A ALL EXCAVATION SHALL COMPLY WITH OSHA STANDARDS
- B. DEMOLISH AND REMOVE ALL OBSTRUCTIONS AS REQUIRED. AREAS TO RECEIVE FOUNDATION SHALL BE STRIPPED TO REMOVE ALL ORGANIC MATERIAL AND CONTAMINATED OR SOFT SOIL. DISPOSAL OF ALL DEBRIS FROM DEMOLITION AND STRIPPING OPERATIONS SHALL BE AS SPECIFIED BY OWNER.
- C. CARE SHALL BE TAKEN TO NOT OVER EXCAVATE BELOW BOTTOM OF FOUNDATION. ANY OVER EXCAVATION REQUIRED FOR REMOVAL OF THE EXISTING FILL SHALL BE BACKFILLED PER SECTION 2.A OR 2.B
- D. ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY PROOF-ROLLING PER TX DOT ITEM 216 TO DETERMINE THAT ALL LOOSE, SOFT, OR OTHERWISE UNDESIRABLE MATERIALS ARE REMOVED. IF AN AREA OF UNDESIRABLE MATERIAL IS DISCOVERED AT THE BOTTOM OF THE EXCAVATION, IT SHALL BE REMOVED AND REPLACED WITH COMPACTED BACKFILL PER SECTION 2.A. OR 2.B.
- E. THE UPPER 6" OF EXPOSED SOILS SHALL BE COMPACTED TO AT LEAST 95% OF MAX. DRY DENSITY DETERMINED BY MODIFIED PROCTOR TEST (ASTM D1557)
- F. WHERE SOIL CONDITIONS PERMIT, FOUNDATIONS BELOW GRADE MAY BE EARTH FORMED UNLESS OTHERWISE NOTED.

2. ENGINEERED FILL (CLAY)

- A. CLAY STRUCTURE FILL SHALL BE SANDY CLAY WITH LIQUID LIMIT OF LESS THAN 35 AND PLASTICITY INDEX (PI) BETWEEN 8 AND 20.
- B. CLAY SHALL BE MOISTURE CONDITIONED WITHIN 2% OF OPTIMUM MOISTURE CONTENT AND COMPACTED TO AT LEAST 95 % OF THE MAX DRY DENSITY DETERMINED BY THE MODIFIED PROCTOR TEST (ASTM D1557) WITH 8" MAX. LOOSE LIFTS.
- C. FOR COMPACTION BY MANUALLY-GUIDED POWER COMPACTORS, STRUCTURAL FILL SHALL BE PLACED IN LIFTS OF 6" MAXIMUM LOOSE THICKNESS.

NOTES ON PRESSURE TREATED LUMBER:

GALVANIZED

FASTENERS (AND OTHER METAL PRODUCTS) FOR USE WITH WOOD TREATED WITH ACQ PRESERVATIVES INCLUDE: HOT-DIP GALVANIZED (THE MINIMUM STANDARD) THE MINIMUM HOT-DIP GALVANIZED REQUIREMENT FOR USE WITH TREATED WOOD SHOULD CONFORM TO THE FOLLOWING ASTM STANDARDS: ASTM- A153 (FOR HOT-DIP FASTENER PRODUCTS) AND ASTM-A653 (COATING DESIGNATION g-185 FOR HOT-DIP CONNECTOR AND SHEET PRODUCTS).

STAINLESS STEEL STAINLESS STEEL FASTENERS AND CONNECTORS ARE REQUIRED FOR PERMANENT WOOD FOUNDATIONS BELOW GRADE AND ARE RECOMMENDED FOR USE WITH TREATED WOOD IN OTHER SEVERE EXTERIOR APPLICATIONS SUCH AS SWIMMING POOLS, SALT WATER EXPOSURE, ETC. - TYPE 304 AND 316 ARE THE RECOMMENDED GRADES TO US

ALUMINUM SHOULD NOT BE USED IN DIRECT CONTACT WITH PRODUCTS TREATED WITH ACQ PRESERVATIVE SPACER MATERIALS OR OTHER PHYSICAL BARRIERS ARE RECOMMENDED TO PREVENT DIRECT CONTACT OF ACQ TREATED WOOD WITH ALUMINUM PRODUCTS.

STRUCTURAL:

IF TRUSSES ARE SPECIFIED ON THE PLANS. THE TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND/OR STRESS AND LOAD CALCULATIONS (DIAGRAMS) FOR CONTRACTORS APPROVAL PRIOR TO CONSTRUCTION. DRAWINGS SHALL BEAR SEAL OF THE REGISTERED ENGINEER IN THE STATE IN WHICH THE STRUCTURE IS BUILT.

MISC. BOLTS AND THREADED FASTENERS

- A. SPECIFICATION
- B. DESIGN
- C. INSTALLATION

1603.1.4 WIND DESIGN DATA:

THE FOLLOWING INFORMATION RELATED TO WIND LOADS SHALL BE SHOWN, REGARDLESS OF WHETHER WIND LOADS GOVERN THE DESIGN OF THE LATERAL FORCE-RESISTING SYSTEM OF THE STRUCTURE:

- RISK CATEGORY. 2.
- UTILIZED

ALL WOOD MEMBERS (INCLUDING PLYWOOD SHEATHING & ALL WOOD BASED MATERIALS) IN CONTACT WITH CONCRETE. OR EXPOSED TO WEATHER. MOISTURE OR WITHIN 18" OF THE GROUND (SUCH AS PORCH & BALCONY FRAMING) SHALL BE PRESSURE- TREATED.

CURRENTLY, THE PRODUCT COMMONLY USED FOR PRESSURE TREATMENT IS ALKALINE COPPER QUATERNARY (ACQ). THIS MATERIAL IS EXTREMELY CORROSIVE. ONLY HOT- DIPPED

ANCHOR BOLTS, THRU BOLTS, NAILS, OR OTHER CORROSIVE-RESISTANT FASTENERS, SHALL BE USED WITH ACQ-TREATED LUMBER. FASTENER MANUFACTURER OR SUPPLIER SHALL BE CONSULTED ON THE SUITABILITY OF GALVANIZED FASTENERS FOR USE WITH TREATED LUMBER.

1. BOLTS SHALL CONFIRM TO ASTM A307 GRADE A. CARBON STEEL EXTERNALLY THREADEDFASTENERS, U.N.O.

2. BOLTS AND NUTS SHALL BE HEX HEAD ASTM A307 AND CONFORM TO ANSI STANDARDS B18.2.1 AND B18.2.1 AS WELL AS ASTM MATERIAL STANDARDS ASTM 307. 3. WASHERS SHALL BE CIRCULAR, FLAT AND SMOOTH IN CONFORMANCE WITH THE REQUIREMENTS OF TYPE A WASHERS IN ANSI STANDARD B23.1.

1. THE MINIMUM BOLT DIAMETER SHALL BE $\frac{1}{2}$ INCH AND BE BEARING TYPE CONNECTION USING STANDARD HOLES WITH THREADS EXCLUDED FROM THE SHEAR PLANE, U.N.O.

1. A307 BOLTS SHALL BE TIGHTENED PER TURN-OF-NUT BOLTING METHOD. THE TURN-OF-NUT BOLTING METHOD CONDITION DEFINED AS THE TIGHTNESS ATTAINED BY THE EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH, THEN BACKED OFF 2/3 TURN. (PER AISC MANUEL OF STEEL CONSTRUCTION, LOAD & RESISTANCE FACTOR 3RD EDITION 8.2.1) DO NOT OVERTIGHTEN BOLTS AS TO DAMAGE THE WOOD ELEMENTS.

2. BOLTS AND NUTS SHALL BE WELL LUBRICATED AT TIME OF INSTALLATION, DRY, CORRODED BOLTS WILL NOT BE ALLOWED.

3. ALL BOLTS SHALL BE NEW AND NOT REUSED.

4. BOLTED CONNECTIONS SHALL BE KNURLED OR SPOT-WELDED TO PREVENT BACK-OUT.

BASIC DESIGN WIND SPEED, V, MILES PER HOUR AND ALLOWABLE STRESS DESIGN WIND SPEED, VASD, AS DETERMINED IN ACCORDANCE WITH SECTION 1609.3.1.

3. WIND EXPOSURE. APPLICABLE WIND DIRECTION IF MORE THAN ONE WIND EXPOSURE IS

4. APPLICABLE INTERNAL PRESSURE COEFFICIENT.

DESIGN WIND PRESSURES TO BE USED FOR EXTERIOR COMPONENT AND CLADDING MATERIALS NOT SPECIFICALLY DESIGNED BY THE REGISTERED DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN OF THE STRUCTURE, PSF (KN/M2).

THIS STRUCTURE IS IN A WIND COASTAL MEAN HIGH WATER LI GREATER WIND ZONE. OR

IN AN AREA WHERE THE ULTIMA

WIND BORNE DEBRIS PROTECTI OPENING BEING COVERED.

CODES & DESIGN LOADS

CODE: FLORIDA BUILDING CODE- 2020

ZONE LEGEND

(MIN.) DESIGN PRESSURE FOR

ALL AREAS NOT DESIGNATED A ZONE 5 AS INDICATED ARE ZON

WINDOWS, DOORS AND WALLS THE ALPHA VALUE DEFINES THE SIZE OF ZONE 5 (MEASURED FR ANY OUTSIDE CORNER)

SHOULD BE 10% OF α = 3'-0" < VERIFY WITH PLANS

| | COMPONE | N | |
|------|---------|---------|---|
| ZONE | DOOR | WINDOW | 1 |
| 1 | | | |
| 2 | | | |
| 3 | | | |
| 4 | +32/-34 | +34/-36 | |
| 5 | +32/-41 | +34/-45 | |
| ALL | | | |

EXPOSURE = C

INTERNAL PRESSURE COEFFICI PRESSURE AREA 10 SQ. FT.

DESIGN LOADS:

1. ROOF LIVE LOADS 20 PSF (2. FLOOR LIVE LOADS

| USE | | LO |
|------|-----------------------|------|
| EXT | ERIOR BALCONIES | |
| DEC | KS | |
| FIRE | E ESCAPES | |
| STA | IRS/RAMPS | |
| GUA | ARDRAILS & HANDRAILS | |
| (a) | ELEVATED GARAGE FLC | |
| (b) | NO STORAGE LOAD IS F | REQ |
| (c) | INDIVIDUAL STAIR TREA | DS |
| | UNIFORMLY DISTRIBUTE | ED I |
| | CONCENTRATED LOAD | AC |
| | INCHES, WHICHEVER PR | ROE |

(d) A SINGLE CONCENTRATED

POINT ALONG THE TOP.

3. WIND LOADS (AMERICAN SO BASIC WIND DESIGN VELOCITY

4. DEAD LOAD

| USE | LOAD (|
|-------|--------|
| ROOF | 10 |
| DECKS | 10 |
| FLOOR | 10 |
| WALLS | 11 |
| | • |

| | WALL TYPE |
|---------|--------------|
| 1 STORY | EXTERIOR WA |
| HOUSE | INTERIOR WAI |
| | PLUMBING WA |
| | |

NOTES:

1. WALL SHEATHING SHALL BE EXTERIOR COVERINGS SHALL C

DIMENSIONS: 1. DRAWING DIMENSIONS GOVE

SELECTED DOORS, WINDOW BEGINS.

| FASTENER CORRO | ç |
|-------------------------|---|
| MOISTURE EXPOSURE | |
| EXTERIOR | |
| ENCLOSED BUT VENTILATED | |
| AIR CONDITIONED SPACE | |

FLORID

| | DRAWING LIST | | |
|--|--|--|--|
| ORIDA WIND BORNE DEBRIS PROTECTION | TABLE OF CONTENTS XX-XXX-GN-1.00 GENERAL NOTES | | |
| ND BORNE DEBRIS PROTECTION AREAS. IT IS WITHIN 1 MILE OF THE R LINE WHERE THE ULTIMATE WIND SPEED, Vult. AND IS 130MPH OR | XX-XXXX-S-1.00FOUNDATION & CEILING JOIST PLANSXX-XXXX-S-2.00RAFTER AND SHEARWALL PLANS | | |
| IMATE DESIGN WIND SPEED, Vult. IS 140MPH OR GREATER. | XX-XXXX-F-1.00FOUNDATION DETAILSXX-XXXX-SD-1.00STANDARD DETAILS 1 | | |
| ECTIONS SHALL MATCH OR EXCEED THE DESIGN PRESSURE FOR THE | XX-XXX-SD-2.00 STANDARD DETAILS 2 | | |
| | | | |
| 020 (CITY OF FORT PIERCE REQUIREMENTS) | | | |
| | | | |
| OR WINDSTORM COMPLIANCE) | | | |
| D AS ZONE 4 | LEGEND | | |
| <u>LS</u> THE | | | |
| FROM | DETAIL OR SECTION CALLOUT | | |
| OF SHORTEST WALL NS | XX/XX-X.XX | | |
| NENT DESIGN PRESSURE (PSF) N WALL COVERING GARAGE DOOR ROOF | | | |
| +19/-30 +19/-53 | APP. ANTHONY POWER BEAM APP. ANTHONY POWER PRESERVED CANT. CANTILEVER | | |
| +19/-79 | CONT. CONTINUOUS DF. DOUGLAS FIR | | |
| +25/-27 +28/-34 | FND. FOUNDATION F.V. FIELD VERIFY | | |
| FICIENT = 0.18 | HDR. HEADER PLCS. PLACES | | |
| | SIM. SIMILAR S.F. STEEL FLITCH SPE SPRUCE PINE FIR | | |
| F (SUBJECT TO SLOPE & TRIBUTARY AREA REDUCTION FACTORS) | SYP. SOUTHERN YELLOW PINE TRPL. TRIPLE | | |
| LOAD (PSF) USE LOAD (PSF) | NOTES | | |
| 40 SLEEPING ROOMS 30 40 OTHER ROOMS 40 | | | |
| 40 ATTIC W/STORAGE 20 (b) 40 (c) ATTIC W/O STORAGE 10 (b) 250(d) CARACE N/A(a) | 1. ALL SPECIFICATIONS ARE MINIMUM REQUIREMENTS. SPECIFICATIONS MAY BE INCREASED TO MEET ARCHITECTURAL OR | | |
| OORS SHALL BE CAPABLE OF SUPPORTING A | CONSTRUCTION PREFERENCES. | | |
| REQUIRED WITH ROOF SLOPES OF 3 IN 12, OR | Revisions: | | |
| ADS SHALL BE DESIGNED FOR THE ED LIVE LOAD OR A 300-POUNDS ACTING OVER AN AREA OF 4 SOLIARE | # DATE DESCRIPTION OF CHANGE | | |
| RODUCES THE GREATER STRESSES. TED LOAD APPLIED IN ANY DIRECTION @ ANY | | | |
| SOCIETY OF CIVIL ENGINEERS, 7-16) | ALL RIGHTS RESERVED. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR | | |
| TY 160 MPH (ULTIMATE WIND SPEED) RISK CATEGORY: II | UTILIZED IN ANY FORM WITHOUT PRIOR WRITTEN AUTHORIZATION OF COBALT . | | |
| | THIS DRAWING IS A CONCEPTUAL PLAN AND IS NOT | | |
| AD (PSF) 10 12 | INTENDED TO BE USED AS A SITE SPECIFIC PLAN NOR SHALL IT BE USED FOR PERMITTING PURPOSES. SITE SPECIFICS SHALL BE PREPARED ON A CASE BY CASE | | |
| 10 10 11 | BASIS AND ANY USE BEYOND THE AFOREMENTIONED INTENT IS PROHIBITED. | | |
| MINIMUM WALL FRAMING REQUIREMENTS | | | |
| YPEMATERIAL & SPACINGANCHOR BOLT REQUIREMENTSX WALLS2X6 SYP #2 @ 16" O.C.5% "X10" J-BOLTS W/ 3" SQ. X | | | |
| WALLS 2X4 SYP #2 @ 16" O.C. | | | |
| 3 WALLS 2X6 SYP #2 @ 16" O.C. | - | | |
| BE AS INDICATED ON SHEARWALL & HOLDOWN PLAN. LL CONFORM TO FBC INSTALLATION INSTRUCTIONS. | | | |
| OVERN OVER SCALE. VERIFY ALL ROUGH OPENING DIMENSIONS FOR OWS AND MECHANICAL REQUIREMENTS BEFORE CONSTRUCTION | | | |
| ROSION RESISTANCE | | | |
| FINISH | | | |
| HOT DIPPED GALV. (MIN.) | | | |
| ELECTRO GALVANIZED (MIN.) | | | |
| | DRAWN BY:KEPCHECKED BY:CCHPROJECT #:SCALE:N.T.S. | | |
| | DATE: GN-1.00 | | |

| NAILING | SCHEDULE | | | | |
|---|---------------------------|------------------------|---------------------|--|--|
| JOINT DESCRIPTION | NUMBER OF COMMON NAILS | NUMBER OF BOX NAILS | NAIL SPACING | | |
| WALL FRAMING | | | | | |
| Top plate to top plate (face-nailed) | 2-16d | 2-16d | per foot | | |
| Top plates at intersections (face-nailed) | 4-16d | 5-16d | joints - each side | | |
| Stud to stud (face-nailed) | 2-16d | 2-16d | 24"o.c. | | |
| Header to header (face-nailed) | 16d | 16d | 16"o.c. along edges | | |
| Top or bottom plate to stud (end-nailed) | 1 -16d | 1 -40d | per stud | | |
| Bottom plate to floor joist, bandjoist, endjoist or blocking (face-nail | ed) 2-16d | 2-16d | per foot | | |
| FLOOR FRA | MING | | | | |
| Joist to sill, top plate or girder (toe-nailed) | 4-8d | 4-10d | per joist | | |
| Bridging to joist (toe-nailed) | 2-8d | 2-10d | each end | | |
| Blocking to joist (toe-nailed) | 2-8d | 2-10d | each end | | |
| Blocking to sill or top plate (toe-nailed) | 3-16d | 4-16d | each block | | |
| Ledger strip to beam (face-nailed) | 3-16d | 4-16d | each joist | | |
| Joist on ledger to beam (toe-nailed) | 3-8d | 3-10d | per joist | | |
| Band joist to joist (end-nailed) | 3-16d | 4-16d | per joist | | |
| Band joist to sill or top plate (toe-nailed) | 2-16d | 3-16d | per foot | | |
| CEILING SHE | ATHING | | | | |
| Gypsum wallboard | 5d coolers | 5d coolers | 7"edge/10"field | | |
| ROOF FRA | MING | | | | |
| Rafter to top plate (toe-nailed) | 5 -8d | 5 -10d | per rafter | | |
| Ceiling joist to top plate (toe-nailed) | 5 -8d | 5 -10d | per joist | | |
| Ceiling joist to parallel rafter (face-nailed) | 7 -16d | 7 -40d | each lap | | |
| Ceiling joist laps over partitions (face-nailed) | 7 -16d | 7 -40d | each lap | | |
| Collar tie to rafter (face-nailed) | 3 -8d | 3 -10d | per tie | | |
| Blocking to rafter (toe-nailed) | 2-8d | 2 -10d | each end | | |
| Rim board to rafter (end-nailed) | 2-16d | 3 -16d | each end | | |

| r. | | | | | | | |
|----|--|---|--|--------------------|--|--|--|
| | CONCRETE COVERAGE FOR CAST-IN-PLACE (NON PRE-STRESSED CONCRETE MEMBERS) | | | | | | |
| | CONCRETE EXPOSURE | MEMBER | REINFORCEMENT | SPECIFIED COVER | | | |
| | CAST AGAINST AND PERMANENTLY IN CONTACT W/ GROUND | ALL | ALL | 3" | | | |
| | EXPOSED TO WEATHER | ALL SLABS, JOISTS & WALLS | NO. 6 THROUGH NO. 18 BARS | 2" | | | |
| | WITH GROUND | | NO. 5, W31 / D31 WIRE & SMALLER | 1-1⁄2" | | | |
| | | | NO. 14 & NO. 18 BARS | 1-1⁄2" | | | |
| | | | NO. 11 BARS & SMALLER | ³ ⁄4" | | | |
| | CONTACT WITH GROUND | BEAMS, COLUMNS, PEDESTALS, & TENSION TIES | PRIMARY REINFORCEMENT, STIRRUPS, TIES, SPIRALS, & HOOPS | 1-1⁄2" | | | |

| | DEVELOPMENT LENGTH CHART | | | | | | | |
|------------|--------------------------|-------------------------------|---------------|--------|-------|--|--|--|
| SLAB/ MATS | | | | WA | LLS | | | |
| BAR | 12" THICKNESS OR LESS | THICKNESS GREATER THAN 12" | | HORIZ. | VERT. | | | |
| | ALL BARS BOTT BAR | | OTHER BARS | | | | | |
| #3 | 17" | 17" | 22" | 17" | 22" | | | |
| #4 | 22" | 22" | 30" | 22" | 30" | | | |
| #5 | 29" | 29" | 36" | 29" | 36" | | | |

CONSTRUCTIV

| R | evisions: | | | |
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| | HIS DRAW | ING IS A CONC | EPTUAL PLAN AND IS | |
| SF | ALL IT BE | USED FOR PER | RMITTING PURPOSES | 5. SITE CASE |
| BA | ASIS AND A | NY USE BEYON INTENT IS P | ND THE AFOREMENT ROHIBITED. | IONED |
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STRINGER PLAN (STANDARD)

ALL OPTIONS

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| | ENDED IC |) BE USED AS / | A SITE SPECIFIC I | |
| SH/ | ALL II BE | USED FOR PEF | RMITTING PURPO | SES. SITE |
| SP | ECIFICS S | HALL BE PREP | ARED ON A CASE | -BY-CASE |
| BA | BASIS AND ANY USE BEYOND THE AFOREMENTIONED | | | |
| | INTENT IS PROHIBITED. | | | |
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STRINGERS TO CMU PIER W/ SIMPSON HDU4-SDS2.5 HOLD-DOWN (ANCHOR BOLT MAY BE CAST IN PLACE W/ CAPATURED WASHER OR DRILLED & EPOXIED W/ SIMPSON SET-XP EPOXY IN $\frac{3}{4}$ "Ø HOLE *

| | LEGEND |
|---------------|--|
| X XX | DETAIL PAGE # |
| TYPE X Pxx | TYPE OF NOTCH SIZE OF PILING (P06= 6x6) FOR PILING DEPTH DETAIL #6 DWG. SD-1.00 |
| Cxx | (SEE UNLESS NOTED OTHERWISE ON PLAN) SIZE OF CMU BLOCK (C12= 12X12) (SEE SECTIONS A & B THIS DWG. FOR PIER CONSTRUCTION SECTIONS) UNLESS NOTED OTHERWISE ON PLAN) |
| | |
| | |

NOTES

CONCRETE HOLD-DOWNS: 1. SEE DRAWING S-6.00 FOR TYPE OF HOLD-DOWN REQUIRED.

2. SEE DRAWING S-1.00 FOR HOLD-DOWN INSTALLATION DETAIL. (REFER TO MANUFACTURERS INSTALLATION INSTRUCTIONS FOR ADDITIONAL REQ'S.)

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FLOOR JOIST PLAN (STANDARD) 03-

ALL OPTIONS

| VVPK | HA MEMRER SIZE | | | | LEG | END | |
|-----------|--------------------------------------|---|--------------------|---|---|--|---|
| 5 | 2X10 | LUS210 OR LUC210Z CONCEAL | | | | | |
| 6 | (2) 2X10 | HUS210-2 OR HUC210-2 CONCEA | LED | | | | |
| H RECO | OT DIPPED GAL OMMENDED ON EXPO | VANIZED OR STAINLESS STEEL EXTERIOR HANGERS WITH DIREC SURE TO MOISTURE | т | | | | |
| SC | ME MODELS M | AY NOT BE USED (SEE PLAN FOR LOCATION) | | | | | |
| | | | | | | | |
| | | | | | NO | TES | |
| | | | FL0 1. | DOR FRAM HOUSE 16" O C | /ING: E FLOOR JOIS C. (U.N.O.) | TS SHALL BE 2X | (12 #2 SYP @ |
| | | | 2. | DECK I 16" O.C | FLOOR JOISTS C. (U.N.O.) | S SHALL BE 2X1 | 0 #2 SYP @ |
| | | | 3. | Doubl Above | E THE FLOOF RUN PARALL | R JOIST WHERE LEL TO FLOOR J | WALLS OIST. |
| | | | 4. | FLOOR 0.120 x DIAPHI PATTE | R DECKING: ¾ 3" NAILS, 6" A RAGM. 6" EDG RN. (SEE DET | " APA RATED PL AROUND PERIMI E & 12" FIELD N AIL #3 DWG. SD | YWOOD ETER OF AIL -2.00) |
| | | | FR. 1. | AMING: ALL ST ETC. S DIREC | UDS, FLOOR HALL VERTIC, TLY ON EACH | JOISTS, RAFTER ALLY ALIGN ANI OTHER. | RS, TRUSS, D BEAR |
| | | | # | DATE | DE | SCRIPTION OF CHAN | IGE |
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| | | | ALL UTILI | RIGHTS RESER\ ZED IN ANY FO | /ED. NO PART OF TH RM WITHOUT PRIOR | IS DOCUMENT MAY BE F WRITTEN AUTHORIZATI | REPRODUCED OR ON OF "COBALT". |
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| | | | DRAW | 'N BY: | AW | CHECKED BY: | ССН |
| | | | PROJI DATE: | ECT #: | | SCALE: | 1/4" = 1'-0" S-3.00 |

S-4.00

LEGEND

BRACE POINT

BW - BRACE TO WALL BJ - BRACE TO JOIST BB - BRACE TO BEAM BS - BRACE TO STRONGBACK

NOTES

ROOF FRAMING: 1. RIDGES TO BE 2x10 #2 SYP

- 2. ALL HIPS TO BE 2x8 #2 SYP
- 3. ALL RAFTERS TO BE 2x6 #2 SYP 16"O.C. (UNO)
- ROOF SHEATHING: ¹⁹/₃₂" APA RATED PLYWOOD W/ 0.120 X 3" RING SHANK NAILS, 4" AROUND PERIMETER OF DIAPHRAGM. 6" EDGE & 6" FIELD NAIL PATTERN. PER SECTION R803
- 5. ROOF UNDERLAYMENT SHALL COMPLY WITH R905.1.1.1 & TABLE R905.1.1.1

FRAMING:

1. ALL STUDS, FLOOR JOISTS, RAFTERS, TRUSS, ETC. SHALL VERTICALLY ALIGN AND BEAR DIRECTLY ON EACH OTHER.

| R | evisions: | |
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| PROJECT #: | | SCALE: | 1/4" = 1'-0" |
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SHEAR WALLS: ALL SHEARWALLS SHALL EXTEND TO ROOF DIAPHRAGM OR NEXT FLOOR DIAPHRAGM.

SW1: 19/32" APA RATED PLYWOOD SHEATHING 6" PERIMETER AND W/6" EDGE / 12" FIELD NAIL PATTERN USING 0.120 x 3" NAILS

SW4/

SW4: 19/32" APA RATED PLYWOOD SHEATHING 4" PERIMETER AND W/4" EDGE / 12" FIELD NAIL PATTERN USING 0.120 x 3" NAILS

HOLD-DOWNS:

ALL HOLD-DOWNS SHALL BE SECURED TO MIN. DBL STUD OR GREATER PER MANUFACTURERS INSTRUCTIONS.

MSTC48B3 W/ (16) 10D NAILS IN BEAM & 1 (38) 10D NAILS IN STUD PACK PER MANUFACTURERS INSTRUCTIONS

MSTC66B3 W/ (18) 10D NAILS IN BEAM & (38) 10D NAILS IN STUD PACK PERMANUFACTURERS INSTRUCTIONS

HDU5-SDS2.5 USE (14) ¹/₄"X2¹/₂"SDS SCREWS $\frac{3}{3}$ PER MANUFACTURERS INSTRUCTIONS (SEE HDU TO FLOOR BEAM CONNECTION DETAIL ON THIS DWG. FOR INSTALLATION TO FLOOR BEAM).

★ DRAG STRUT BEAM W/ SIMP. HGA10KT AT EACH END (SEE DETAIL S-4.00)

| R | evisions: | |
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HDU TO FLOOR BEAM CONNECTION DETAIL

| DRAWN BY: | AW | CHECKED BY: | ССН |
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| R | evisions: | |
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SCALE:

| KING AND JACK STUD SCHEDULE | | | | |
|-----------------------------|---|--|--|--|
| OPENING SPAN | KING/JACK STUDS REQUIRED PER OPENING SPAN | | | |
| 4' OR LESS | (1) KING & (1) JACK STUD (EACH SIDE) | | | |
| 4' TO 6' OPENING | (2) KING & (2) JACK STUD (EACH SIDE) | | | |
| 6' TO 8' OPENING | (3) KING & (2) JACK STUD (EACH SIDE) | | | |
| 8' TO 12' OPENING | (4) KING & (3) JACK STUD (EACH SIDE) | | | |
| 12' TO 16' OPENING | (5) KING & (4) JACK STUD (EACH SIDE) | | | |

| BEND | MSTA3 |
|------|---------|
| BACK | SIDE C |
| PACK | (6" DO\ |
| STUD | MIN.) |
| | , |

(2) LSTA24 STRAPS

EACH SIDE OF BEAM

AS SHOWN, (4) TOTAL

(2) TOE-NAILS

STRINGERS PER

2

PLAN

| | SIN | GLE STOR | Y HEADER SCHEDULE | |
|---|--------------|--------------------------------|---|--|
| ALL HEADERS ARE DROP BEAMS, U.N.O GRADE # 2 | | | | |
| TYPE | OPENING SPAN | WALL HEADER REQ'D THICKNESS | | |
| 1 | 3' OR LESS | 3.5" | TWO 2"x8" #2 SYP w/ 1/2" PLYWD FLITCH | |
| 2 | 3' OR LESS | 5.5" | THREE 2"x8" #2 SYP w/ 1/2" PLYWD FLITCHS | |
| 3 | 3' TO 5' | 3.5" | TWO 2"x10" #2SYP w/ 1/2" PLYWD FLITCH | |
| 4 | 3' TO 5' | 5.5" | THREE 2"x10" #2SYP w/ 1/2" PLYWD FLITCHS | |
| 5 | 5' TO 8' | 3.5" | TWO 2"x12" #2 SYP w/ 1/2" PLYWD FLITCH | |
| 6 | 5' TO 8' | 5.5" | THREE 2"x12" #2 SYP w/ 1/2" PLYWD FLITCHS | |
| | | • | | |

OPENINGS GREATER THAN 6'-1" SHALL BE AS INDICATED ON PLAN DRAWINGS NOTES:

THIS TABLE USES EITHER 0.25" DEFLECTION OR L/240 WHICHEVER IS LESS

2

THIS TABLE ACCOUNTS FOR ONLY ROOF AND CEILING LOADING

FOR GENERIC 1 STORY FRAMING 3.

FOR 2-STORY DWELLINGS USE 2X12 HEADERS ON FIRST FLOOR LIVING SPACE (UNO) 4.

SLAB ON GRADE

TYPICAL DOOR/ WINDOW FRAMING DETAILS

| 2x6 RAIL CAP 2x4 TOP & BOTTOM RAILS ATTACHED TO GUARD POST W/(2)8d THREADED NAILS OR (2)#8 SCREWS 2 ½" LONG ON INSIDE FACE 2x2 BALUSTERS @ 5" O.C. MAX. ATTACH TO TOP AND BOTTOM RAILS W/(1)#8 WOOD SCREW OR (2)8d (0.135"0) THREADED NAILS W/(1)#8 WOOD SCREW OR (2)8d (0.135"0) THREADED NAILS W/(1)#8 WOOD SCREW OR (2)8d (0.135"0) THREADED NAILS MIN. 4x4 POST @ 6'-0"O.C. MA ATTACHED TO DECK JOIST W/(2 ½" GALV. THRU-BOLTS. DO NOT NOTCH POST BLOCKING ON EACH SIDE OF POST DECK JOISTS PER PLAN (3) ½" GALV. THRU-BOLTS (H.D.G.) BAND JOIST PER PLAN | | | | | |
|---|--------------------------------|---|--|---|---|
| N BLOCKING TOGETHER WITH (12) NAILS (0.131"x2-1/2") | | | | | |
| DECK RAIL INSTALLATION DETAIL | R # # ALL F UTILIZ | evisions: DATE DATE IN ANY FOR TENDED TO ALL IT BE PECIFICS S ASIS AND A | ING IS A CONC DE USED AS USED FOR PEF HALL BE PREP NY USE BEYOI INTENT IS P | EPTUAL PLAN A SITE SPECIFIC RMITTING PURP ARED ON A CAS ND THE AFOREM ROHIBITED. | ANGE E REPRODUCED OR ATION OF "COBALT". |
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| | 5 | 9 | 1 | 10 | 1 | 1 | | 12 | | 13 | | 4 | 1 | 5 | 1 | .6 |
|-----|----|-----|----|-----|----|-----|----|-----|----|---------|----|-----|----|-----|----|-----|
| l0d | 8d | 10d | 8d | 10d | 8d | 10d | 8d | 10d |
| 16 | 28 | 18 | 32 | 20 | 34 | 22 | 37 | 24 | 40 | 26 | 43 | 28 | 47 | 30 | 50 | 32 |
| | | | | | | | | | | · · · · | | | | · · | | |

| | Ş | 9 | | 10 | 1 | 1 | 1 | 2 | | 13 | 1 | 14 | 1 | 15 | 1 | 6 |
|----|----|-----|----|-----|----|-----|----|-----|----|-----|----|-----|----|-----|----|-----|
| 0d | 8d | 10d |
| 20 | 34 | 24 | 38 | 28 | 42 | 30 | 46 | 32 | 50 | 35 | 53 | 38 | 57 | 40 | 61 | 43 |

| | 9 | | | 10 | 11 | | 12 | | 13 | | 1 | 14 1 | | 15 16 | | |
|----|----|-----|----|-----|----|-----|----|-----|----|-----|----|------|----|-------|----|-----|
| 0d | 8d | 10d | 8d | 10d | 8d | 10d |
| 24 | 42 | 28 | 46 | 32 | 52 | 35 | 56 | 38 | 61 | 41 | 66 | 44 | 70 | 47 | 75 | 50 |

FEMA BREAKAWAY WALL FASTENING TABLE

NOTES

- THIS DRAWING MAY NOT APPLY IF THE 1. STRUCTURE IS NOT IN A FLOOD ZONE.
- FEMA APPROVED FLOOD VENTS ARE 2. REQUIRED WHEN ENCLOSED SPACE IS BELOW B.F.E. IN AN "A" FLOOD ZONE.
- FEMA APPROVED BREAKAWAY WALLS OR 3. LOUVERS ARE REQUIRED WHEN ENCLOSED SPACE IS BELOW B.F.E. IN A "V" FLOOD ZONE. (EXCEPTION: UP TO 299 SQ. FT. MAY BE ENCLOSED W/O BREAKAWAY OR LOUVERED WALLS.)
- ALL MATERIALS BELOW B.F.E. SHALL BE 4. FLOOD DAMAGE RESISTANT MATERIAL, INCLUDING, BUT NOT LIMITED TO TREATED LUMBER. SEE FEMA TECHNICAL BULLETIN #2 FOR ADDITIONAL INFORMATION.

| R | evisions: | |
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| ALL I UTILIZ | RIGHTS RESERV ZED IN ANY FOR | ED. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR RAM WITHOUT PRIOR WRITTEN AUTHORIZATION OF "COBALT". |

THIS DRAWING IS A CONCEPTUAL PLAN AND IS NOT INTENDED TO BE USED AS A SITE SPECIFIC PLAN NOR SHALL IT BE USED FOR PERMITTING PURPOSES. SITE SPECIFICS SHALL BE PREPARED ON A CASE-BY-CASE BASIS AND ANY USE BEYOND THE AFOREMENTIONED INTENT IS PROHIBITED.

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DESIGN AND CONSTRUCTION:

ALL BREAKAWAY WALLS DESIGNED USING THE PRESCRIPTIVE DESIGN METHOD SHALL BE DETAILED IN ACCORDANCE WITH THE FOLLOWING:

1. BREAKAWAY WALLS SHALL BE DESIGNED TO MEET ALL APPLICABLE LOCAL REQUIREMENTS AND BUILDING CODE REQUIREMENTS.

2. AS SHOWN IN BREAKAWAY WALL SECTION (THIS DWG.), WOOD-FRAMED AND STEEL STUD-FRAMED BREAKAWAY WALL PANELS SHALL NOT BE ATTACHED TO THE PILINGS OR OTHER VERTICAL FOUNDATION MEMBERS. ONLY THE TOPS AND BOTTOMS OF WALL PANELS SHALL BE CONNECTED TO PERMANENT 2X4 NAILER PLATES. HIGH-CAPAC ITY CONNECTORS SUCH AS BOLTS, LAG SCREWS, METAL STRAPS, OR HURRICANE FASTENERS (E.G., CLIPS OR STRAPS) SHALL

3. THE EXTERIOR SHEATHING ON BREAKAWAY WALL PANELS SHALL NEITHER OVERLAP NOR BE ATTACHED TO THE VERTICAL FOUNDATION MEMBERS.

4. BREAKAWAY WALL SHEATHING AND SIDING SHALL BE DISCONTINUOUS AT ELEVATED FLOOR BEAMS AND JOISTS; HORIZONTAL SEPARATION JOINTS SHALL BE PROVIDED TO PREVENT DAMAGE TO THE SHEATHING OR SIDING ABOVE THE FLOOR OF THE ELEVATED BUILDING.

5. UTILITIES, INCLUDING ELECTRICAL WIRING, BREAKER BOXES, POWER METERS, PLUMBING, CONDUITS, AND VENTILATION DUCTS, SHALL NOT BE PLACED IN OR ATTACHED TO BREAKAWAY WALL PANELS.

6. BREAKAWAY WALL PANELS SHALL BE POSITIONED SUCH THAT, ON FAILURE, THEY DO NOT COLLAPSE AGAINST CROSS-BRACING OR THREATEN OTHER FOUNDATION COMPONENTS.*FOR MORE INFORMATION, SEE TECHNICAL

7. PARTIAL HEIGHT BREAKAWAY WALL SYSTEMS ARE NOT

RESPONSIBILITIES:

- 1. GENERAL CONTRACTOR IS RESPONSIBLE FOR VERIFYING STRUCTURAL DRAWINGS & ARCHITECTURAL DRAWINGS ARE IN AGREEMENT. ANY DISCOURSE BETWEEN THE TWO SETS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER & ARCHITECT FOR RESOLUTION.
- 2. GENERAL CONTRACTOR IS RESPONSIBLE FOR VERIFYING STRUCTURAL DRAWINGS TO CONFIRM THE AVAILABILITY OF ALL REQUIRED DETAILS. IF ANY REQUIRED INFORMATION IS NOT LISTED IN THE STRUCTURAL DRAWINGS, GENERAL CONTRACTOR IS REQUIRED TO CONTACT ENGINEER OF RECORD FOR PARTICULAR INFORMATION.
- 3. GENERAL CONTRACTOR IS RESPONSIBLE FOR ENSURING CONSTRUCTION ADHERES TO FEMA FLOOD PLAIN, LOCAL MUNICIPALITY, (FFE) FINISH FLOOR ELEVATION REQUIREMENTS AND/OR DISASTER RECOVERY CONSTRUCTION GUIDELINES.
- 4. GENERAL CONTRACTOR IS RESPONSIBLE FOR COMPLIANCE WITH ALL LOCAL MUNICIPALITY CODE REQUIREMENTS AND/OR DISASTER RECOVERY CONSTRUCTION GUIDELINES
- COBALT ENGINEERING IS NOT RESPONSIBLE FOR DESIGN FLOOD ELEVATION OR ANY REQUIREMENT EXCEEDING THE BASE FLOOD ELEVATION AS SHOWN ON THE PROVIDED ELEVATION CERTIFICATE.
- 6. COBALT ENGINEERING & INSPECTIONS LLC. IS NOT RESPONSIBLE FOR THE PERFORMANCE OF FOUNDATION AS A RESULT OF THE BEHAVIOR OF THE SUPPORTING SOIL AND/OR DIFFERENTIAL SETTLEMENT DUE TO SEASONAL CHANGES SUCH AS DROUGHT. EXTENSIVE RAIN AND OTHER DRASTIC CLIMATE CHANGES.

CONCRETE:

- 1. UNLESS OTHERWISE NOTED, ALL CONCRETE FOUNDATION WALLS AND SLABS ON GRADE SHALL BE 3,000 PSI (28 DAY COMPRESSION STRENGTH) CONCRETE, PLACE CONCRETE SLABS ON 4" OF COMPACTED LOW P.I SAND FILL. ALL SLABS UNDER INTERIOR FINISHED AND HEATED LIVING SPACES SHALL BE PLACED ON 6 MIL POLYETHYLENE VAPOR BARRIER WITH A MINIMUM OF 6" LAPPED JOINTS. ALL REBAR LAP SPACING LENGTHS SHALL BE MINIMUM 50 TIMES THE BAR SIZE.
- 2. PROVIDE ¹/₂" EXPANSION JOINT MATERIAL BETWEEN ALL CONCRETE SLABS ABUTTING CONCRETE OR MASONRY WALLS OCCURRING IN EXTERIOR OR UNHEATED SPACES OR AREAS.
- 3. CONCRETE FOR ALL BASEMENT WALLS, FOUNDATION WALLS, PORCHES, WALKS, PATIOS, STEPS, GARAGE, CARPORT FLOOR SLABS AND DRIVEWAYS SHALL BE AIR-ENTRAINED.
- 4. ALL REINFORCEMENT STEEL SHALL MEET ASTM A615 GRADE 60 SPECIFICATIONS.
- 5. REBAR COVER: PROVIDE THE FOLLOWING CONCRETE PROTECTIVE COVERINGS FOR REINFORCEMENT, UNLESS NOTED OTHERWISE.
- A. 3" FOR ALL CONCRETE DEPOSITED DIRECTLY AGAINST THE GROUND. B. 2" FOR ALL FORMED CONCRETE EXPOSED TO WEATHER OR IN CONTACT WITH THE
- GROUND. (UNLESS NOTED OTHERWISE)
- 6. DEVELOPMENTAL LENGTH (DL) SHALL BE 50 TIMES THE DIAMETER OF THE REBAR

RESPONSIBILITIES:

A. ALL (EARTHWORK, EXCAVATIONS, STRUCTURAL FILL, COMPACTION, ETC) SHALL BE DONE IN ACCORDANCE WITH GEO-TECHNICAL REPORT

1. EXCAVATING

- A ALL EXCAVATION SHALL COMPLY WITH OSHA STANDARDS
- B. DEMOLISH AND REMOVE ALL OBSTRUCTIONS AS REQUIRED. AREAS TO RECEIVE FOUNDATION SHALL BE STRIPPED TO REMOVE ALL ORGANIC MATERIAL AND CONTAMINATED OR SOFT SOIL. DISPOSAL OF ALL DEBRIS FROM DEMOLITION AND STRIPPING OPERATIONS SHALL BE AS SPECIFIED BY OWNER.
- C. CARE SHALL BE TAKEN TO NOT OVER EXCAVATE BELOW BOTTOM OF FOUNDATION. ANY OVER EXCAVATION REQUIRED FOR REMOVAL OF THE EXISTING FILL SHALL BE BACKFILLED PER SECTION 2.A OR 2.B
- D. ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY PROOF-ROLLING PER TX DOT ITEM 216 TO DETERMINE THAT ALL LOOSE, SOFT, OR OTHERWISE UNDESIRABLE MATERIALS ARE REMOVED. IF AN AREA OF UNDESIRABLE MATERIAL IS DISCOVERED AT THE BOTTOM OF THE EXCAVATION, IT SHALL BE REMOVED AND REPLACED WITH COMPACTED BACKFILL PER SECTION 2.A. OR 2.B.
- E. THE UPPER 6" OF EXPOSED SOILS SHALL BE COMPACTED TO AT LEAST 95% OF MAX. DRY DENSITY DETERMINED BY MODIFIED PROCTOR TEST (ASTM D1557)
- F. WHERE SOIL CONDITIONS PERMIT, FOUNDATIONS BELOW GRADE MAY BE EARTH FORMED UNLESS OTHERWISE NOTED.

2. ENGINEERED FILL (CLAY)

- A. CLAY STRUCTURE FILL SHALL BE SANDY CLAY WITH LIQUID LIMIT OF LESS THAN 35 AND PLASTICITY INDEX (PI) BETWEEN 8 AND 20.
- B. CLAY SHALL BE MOISTURE CONDITIONED WITHIN 2% OF OPTIMUM MOISTURE CONTENT AND COMPACTED TO AT LEAST 95 % OF THE MAX DRY DENSITY DETERMINED BY THE MODIFIED PROCTOR TEST (ASTM D1557) WITH 8" MAX. LOOSE LIFTS.
- C. FOR COMPACTION BY MANUALLY-GUIDED POWER COMPACTORS, STRUCTURAL FILL SHALL BE PLACED IN LIFTS OF 6" MAXIMUM LOOSE THICKNESS.

NOTES ON PRESSURE TREATED LUMBER:

GALVANIZED

FASTENERS (AND OTHER METAL PRODUCTS) FOR USE WITH WOOD TREATED WITH ACQ PRESERVATIVES INCLUDE: HOT-DIP GALVANIZED (THE MINIMUM STANDARD) THE MINIMUM HOT-DIP GALVANIZED REQUIREMENT FOR USE WITH TREATED WOOD SHOULD CONFORM TO THE FOLLOWING ASTM STANDARDS: ASTM- A153 (FOR HOT-DIP FASTENER PRODUCTS) AND ASTM-A653 (COATING DESIGNATION g-185 FOR HOT-DIP CONNECTOR AND SHEET PRODUCTS).

STAINLESS STEEL STAINLESS STEEL FASTENERS AND CONNECTORS ARE REQUIRED FOR PERMANENT WOOD FOUNDATIONS BELOW GRADE AND ARE RECOMMENDED FOR USE WITH TREATED WOOD IN OTHER SEVERE EXTERIOR APPLICATIONS SUCH AS SWIMMING POOLS, SALT WATER EXPOSURE, ETC. - TYPE 304 AND 316 ARE THE RECOMMENDED GRADES TO US

ALUMINUM SHOULD NOT BE USED IN DIRECT CONTACT WITH PRODUCTS TREATED WITH ACQ PRESERVATIVE SPACER MATERIALS OR OTHER PHYSICAL BARRIERS ARE RECOMMENDED TO PREVENT DIRECT CONTACT OF ACQ TREATED WOOD WITH ALUMINUM PRODUCTS.

STRUCTURAL:

IF TRUSSES ARE SPECIFIED ON THE PLANS. THE TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND/OR STRESS AND LOAD CALCULATIONS (DIAGRAMS) FOR CONTRACTORS APPROVAL PRIOR TO CONSTRUCTION. DRAWINGS SHALL BEAR SEAL OF THE REGISTERED ENGINEER IN THE STATE IN WHICH THE STRUCTURE IS BUILT.

MISC. BOLTS AND THREADED FASTENERS

- A. SPECIFICATION
- B. DESIGN
- C. INSTALLATION

1603.1.4 WIND DESIGN DATA:

THE FOLLOWING INFORMATION RELATED TO WIND LOADS SHALL BE SHOWN, REGARDLESS OF WHETHER WIND LOADS GOVERN THE DESIGN OF THE LATERAL FORCE-RESISTING SYSTEM OF THE STRUCTURE:

- RISK CATEGORY. 2.
- UTILIZED

ALL WOOD MEMBERS (INCLUDING PLYWOOD SHEATHING & ALL WOOD BASED MATERIALS) IN CONTACT WITH CONCRETE. OR EXPOSED TO WEATHER. MOISTURE OR WITHIN 18" OF THE GROUND (SUCH AS PORCH & BALCONY FRAMING) SHALL BE PRESSURE- TREATED.

CURRENTLY, THE PRODUCT COMMONLY USED FOR PRESSURE TREATMENT IS ALKALINE COPPER QUATERNARY (ACQ). THIS MATERIAL IS EXTREMELY CORROSIVE. ONLY HOT- DIPPED

ANCHOR BOLTS, THRU BOLTS, NAILS, OR OTHER CORROSIVE-RESISTANT FASTENERS, SHALL BE USED WITH ACQ-TREATED LUMBER. FASTENER MANUFACTURER OR SUPPLIER SHALL BE CONSULTED ON THE SUITABILITY OF GALVANIZED FASTENERS FOR USE WITH TREATED LUMBER.

1. BOLTS SHALL CONFIRM TO ASTM A307 GRADE A. CARBON STEEL EXTERNALLY THREADEDFASTENERS, U.N.O.

2. BOLTS AND NUTS SHALL BE HEX HEAD ASTM A307 AND CONFORM TO ANSI STANDARDS B18.2.1 AND B18.2.1 AS WELL AS ASTM MATERIAL STANDARDS ASTM 307. 3. WASHERS SHALL BE CIRCULAR, FLAT AND SMOOTH IN CONFORMANCE WITH THE REQUIREMENTS OF TYPE A WASHERS IN ANSI STANDARD B23.1.

1. THE MINIMUM BOLT DIAMETER SHALL BE $\frac{1}{2}$ INCH AND BE BEARING TYPE CONNECTION USING STANDARD HOLES WITH THREADS EXCLUDED FROM THE SHEAR PLANE, U.N.O.

1. A307 BOLTS SHALL BE TIGHTENED PER TURN-OF-NUT BOLTING METHOD. THE TURN-OF-NUT BOLTING METHOD CONDITION DEFINED AS THE TIGHTNESS ATTAINED BY THE EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH, THEN BACKED OFF 2/3 TURN. (PER AISC MANUEL OF STEEL CONSTRUCTION, LOAD & RESISTANCE FACTOR 3RD EDITION 8.2.1) DO NOT OVERTIGHTEN BOLTS AS TO DAMAGE THE WOOD ELEMENTS.

2. BOLTS AND NUTS SHALL BE WELL LUBRICATED AT TIME OF INSTALLATION, DRY, CORRODED BOLTS WILL NOT BE ALLOWED.

3. ALL BOLTS SHALL BE NEW AND NOT REUSED.

4. BOLTED CONNECTIONS SHALL BE KNURLED OR SPOT-WELDED TO PREVENT BACK-OUT.

BASIC DESIGN WIND SPEED, V, MILES PER HOUR AND ALLOWABLE STRESS DESIGN WIND SPEED, VASD, AS DETERMINED IN ACCORDANCE WITH SECTION 1609.3.1.

3. WIND EXPOSURE. APPLICABLE WIND DIRECTION IF MORE THAN ONE WIND EXPOSURE IS

4. APPLICABLE INTERNAL PRESSURE COEFFICIENT.

DESIGN WIND PRESSURES TO BE USED FOR EXTERIOR COMPONENT AND CLADDING MATERIALS NOT SPECIFICALLY DESIGNED BY THE REGISTERED DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN OF THE STRUCTURE, PSF (KN/M2).

THIS STRUCTURE IS IN A WIND I COASTAL MEAN HIGH WATER LI GREATER WIND ZONE. OR

IN AN AREA WHERE THE ULTIMA

WIND BORNE DEBRIS PROTECTI OPENING BEING COVERED.

CODES & DESIGN LOADS

CODE: FLORIDA BUILDING CODE- 2020

ZONE LEGEND

(MIN.) DESIGN PRESSURE FOR

ALL AREAS NOT DESIGNATED A ZONE 5 AS INDICATED ARE ZON

WINDOWS, DOORS AND WALLS THE ALPHA VALUE DEFINES THE SIZE OF ZONE 5 (MEASURED FR ANY OUTSIDE CORNER)

SHOULD BE 10% OF α = 3'-0" < VERIFY WITH PLANS

| | | COMPONE | N |
|------|---------|---------|---|
| ZONE | DOOR | WINDOW | 1 |
| 1 | | | |
| 2 | | | |
| 3 | | | |
| 4 | +32/-34 | +34/-36 | |
| 5 | +32/-41 | +34/-45 | |
| ALL | | | |

EXPOSURE = C

INTERNAL PRESSURE COEFFICI PRESSURE AREA 10 SQ. FT.

DESIGN LOADS:

1. ROOF LIVE LOADS 20 PSF (2. FLOOR LIVE LOADS

| USE | | LO |
|------|-----------------------|------|
| EXT | ERIOR BALCONIES | |
| DEC | KS | |
| FIRE | E ESCAPES | |
| STA | IRS/RAMPS | |
| GUA | ARDRAILS & HANDRAILS | |
| (a) | ELEVATED GARAGE FLC | |
| (b) | NO STORAGE LOAD IS F | REQ |
| (c) | INDIVIDUAL STAIR TREA | DS |
| | UNIFORMLY DISTRIBUTE | ED I |
| | CONCENTRATED LOAD | AC |
| | INCHES, WHICHEVER PR | ROE |

(d) A SINGLE CONCENTRATED

POINT ALONG THE TOP.

3. WIND LOADS (AMERICAN SO BASIC WIND DESIGN VELOCITY

4. DEAD LOAD

| USE | LOAD (|
|-------|--------|
| ROOF | 10 |
| DECKS | 10 |
| FLOOR | 10 |
| WALLS | 11 |
| | • |

| | WALL TYPE |
|---------|--------------|
| 1 STORY | EXTERIOR WA |
| HOUSE | INTERIOR WAI |
| | PLUMBING WA |
| | |

NOTES:

1. WALL SHEATHING SHALL BE EXTERIOR COVERINGS SHALL C

DIMENSIONS: 1. DRAWING DIMENSIONS GOVE

SELECTED DOORS, WINDOW BEGINS.

| FASTENER CORRO | ç |
|-------------------------|---|
| MOISTURE EXPOSURE | |
| EXTERIOR | |
| ENCLOSED BUT VENTILATED | |
| AIR CONDITIONED SPACE | |

FLORID

| | | | | D | RAWING LIST |
|---|---|---|------------------------------------|--|--|
| ORIDA WIND E | BORNE DEBRIS PROTE | CTION | | TABLE OF CONTEN | I TS GENERAL NOTES 1 |
| <u>ND BORNE DE</u> R LINE WHERI | EBRIS PROTECTION ARE | AS. IT IS WITHIN SPEED, Vult. AND | 1 MILE OF THE IS 130MPH OR | XX-XXXX-GN-2.00 XX-XXXX-F-1.00 XX-XXXX-F-2.00 | GENERAL NOTES 2 FOUNDATION & CMU WALL PLANS FOUNDATION DETAILS |
| IMATE DESIG | SN WIND SPEED, Vult. IS | 140MPH OR GRE | ATER. | XX-XXXX-S-1.00 XX-XXXX-SD-1.00 | CEILING JOIST & RAFTER PLANS STANDARD DETAILS 1 |
| ECTIONS SHA | LL MATCH OR EXCEED | THE DESIGN PRE | SSURE FOR THE | XX-XXXX-SD-2.00 | STANDARD DETAILS 2 |
| 020 (CITY OF | FORT PIERCE REQUIR | EMENTS) | | | |
| OR WINDSTOF | RM COMPLIANCE) | | | | |
| D AS CONE 4 | | | ~ | | |
| <u>LS</u> THE EROM | | | | TYPICAL DE | TAIL OR SECTION CALLOUT |
| | | | | DETAIL | PAGE |
| NS | I WALL | | - | | XX/XX-X.XX |
| NENT DESIGI | N PRESSURE (PSF) VERING GARAGE DOO | R ROOF | _ | ABBREVIA APB | ATIONS ANTHONY POWER BEAM |
| | | +19/-30 +19/-53 | _ | APP. CANT. | ANTHONY POWER PRESERVED CANTILEVER |
| j | | +19/-79 | _ | CONT. DF. | CONTINUOUS DOUGLAS FIR |
| +25/- | -27 +28/-34 | | - | DWG. FND. | DRAWING FOUNDATION |
| | 10 | I | | HDR. PLCS | HEADER PLACES |
| FICIENT = 0. | .18 | | | SIM. S.F. | SIMILAR STEEL FLITCH |
| SF (SUBJECT | TO SLOPE & TRIBUTAR | Y AREA REDUCT | TION FACTORS) | SPF. SYP. TRPL. TYP. | SPRUCE PINE FIR SOUTHERN YELLOW PINE TRIPLE TYPICAL |
| LOAD (PSF) 40 | USE SLEEPING ROOMS | LOAD (PSF) 30 | | | NOTES |
| 40 40 250(d) OORS SHALL PLIED OVER A | OTHER ROOMS ATTIC W/STORAGE ATTIC W/O STORAGE GARAGE BE CAPABLE OF SUPP A 20-SQUARE-INCH ARE | 40 20 (b) 10 (b) N/A(a) ORTING A EA. | | 1. ALL SPECIFI REQUIREME INCREASED CONSTRUC | CATIONS ARE MINIMUM NTS. SPECIFICATIONS MAY BE TO MEET ARCHITECTURAL OR FION PREFERENCES. |
| ADS SHALL BI | E DESIGNED FOR THE | 5 IN 12, OK | | Revisions: # DATE | DESCRIPTION OF CHANGE |
| ED LIVE LOAI ACTING OVE RODUCES TH TED LOAD AF | D OR A 300-POUNDS ER AN AREA OF 4 SQUA HE GREATER STRESSE PPLIED IN ANY DIRECTIO | RE S. ON @ ANY | | | |
| SOCIETY OF TY 160 MPH (I RISK C | CIVIL ENGINEERS, 7-16 ULTIMATE WIND SPEED ATEGORY: II | 5))) | | ALL RIGHTS RESERVED. NO UTILIZED IN ANY FORM WITH | PART OF THIS DOCUMENT MAY BE REPRODUCED OR HOUT PRIOR WRITTEN AUTHORIZATION OF "COBALT". |
| | | | | | S A CONCEPTUAL PLAN AND IS NOT |
| 10 10 | | | | SHALL IT BE USED | FOR PERMITTING PURPOSES. SITE BE PREPARED ON A CASE-BY-CASE |
| 10 | | | | BASIS AND ANY US INT | SE BEYOND THE AFOREMENTIONED ENT IS PROHIBITED. |
| MINIMUM WA | LL FRAMING REQUIRE | MENTS | | | |
| | ATERIAL & SPACING | ANCHOR BOLT | T REQUIREMENTS DLTS W/ 3" SQ. X | <u> </u> | |
| WALLS 2 WALLS 2 | X4 SYP #2 @ 16" O.C. | .125"THK. WA | SHER @ 32" O.C. | | |
| WALLS 2 | X6 SYP #2 @ 16" O.C. | 1 | | | |
| BE AS INDICA LL CONFORM | ATED ON SHEARWALL & I TO FBC INSTALLATION | & HOLDOWN PLA I INSTRUCTIONS | N. | | |
| OVERN OVEF OWS AND ME | R SCALE. VERIFY ALL R ECHANICAL REQUIREME | OUGH OPENING ENTS BEFORE CO | DIMENSIONS FOR ONSTRUCTION | | |
| ROSION RES | ISTANCE | | | | |
| | FINISH | | | | |
| HOT DI | PPED GALV. (MIN.) | | | | |
| | COATED (MIN.) | | | | |
| | | | | PROJECT #: | SCALE: N.T.S. |
| | | | | DATE: | GN-1.00 |

| NAILING | SCHEDULE | | |
|---|--------------|------------|---------------------|
| JOINT DESCRIPTION | NAIL SPACING | | |
| WALL FRAM | MING | | |
| Top plate to top plate (face-nailed) | 2-16d | 2-16d | per foot |
| Top plates at intersections (face-nailed) | 4-16d | 5-16d | joints - each side |
| Stud to stud (face-nailed) | 2-16d | 2-16d | 24"o.c. |
| Header to header (face-nailed) | 16d | 16d | 16"o.c. along edges |
| Top or bottom plate to stud (end-nailed) | 1 -16d | 1 -40d | per stud |
| Bottom plate to floor joist, bandjoist, endjoist or blocking (face-nail | ed) 2-16d | 2-16d | per foot |
| FLOOR FRA | MING | | |
| Joist to sill, top plate or girder (toe-nailed) | 4-8d | 4-10d | per joist |
| Bridging to joist (toe-nailed) | 2-8d | 2-10d | each end |
| Blocking to joist (toe-nailed) | 2-8d | 2-10d | each end |
| Blocking to sill or top plate (toe-nailed) | 3-16d | 4-16d | each block |
| Ledger strip to beam (face-nailed) | 3-16d | 4-16d | each joist |
| Joist on ledger to beam (toe-nailed) | 3-8d | 3-10d | per joist |
| Band joist to joist (end-nailed) | 3-16d | 4-16d | per joist |
| Band joist to sill or top plate (toe-nailed) | 2-16d | 3-16d | per foot |
| CEILING SHE | ATHING | | |
| Gypsum wallboard | 5d coolers | 5d coolers | 7"edge/10"field |
| ROOF FRA | MING | | |
| Rafter to top plate (toe-nailed) | 5 -8d | 5 -10d | per rafter |
| Ceiling joist to top plate (toe-nailed) | 5 -8d | 5 -10d | per joist |
| Ceiling joist to parallel rafter (face-nailed) | 7 -16d | 7 -40d | each lap |
| Ceiling joist laps over partitions (face-nailed) | 7 -16d | 7 -40d | each lap |
| Collar tie to rafter (face-nailed) | 3 -8d | 3 -10d | per tie |
| Blocking to rafter (toe-nailed) | 2-8d | 2 -10d | each end |
| Rim board to rafter (end-nailed) | 2-16d | 3 -16d | each end |

| CONCRETE COVERAGE FOR CAST-IN-PLACE (NON PRE-STRESSED CONCRETE MEMBERS) | | | | | |
|--|---|--|--------------------|--|--|
| CONCRETE EXPOSURE | MEMBER | REINFORCEMENT | SPECIFIED COVER | | |
| CAST AGAINST AND PERMANENTLY IN CONTACT W/ GROUND | ALL | ALL | 3" | | |
| EXPOSED TO WEATHER OR IN CONTACT WITH GROUND | ALL | NO. 6 THROUGH NO. 18 BARS | 2" | | |
| | | NO. 5, W31 / D31 WIRE & SMALLER | 1-1⁄2" | | |
| NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND | SLABS, | NO. 14 & NO. 18 BARS | 1-1⁄2" | | |
| | WALLS | NO. 11 BARS & SMALLER | ³ ⁄4" | | |
| | BEAMS, COLUMNS, PEDESTALS, & TENSION TIES | PRIMARY REINFORCEMENT, STIRRUPS, TIES, SPIRALS, & HOOPS | 1-1⁄2" | | |

| DEVELOPMENT LENGTH CHART | | | | | |
|--------------------------|--------------------------|--|-----|--------|-------|
| | SLAB/ | | WA | LLS | |
| BAR SIZE | 12" THICKNESS OR LESS | THICKNESS GREATER THAN 12" BOTTOM OTHER BARS BARS | | HORIZ. | VERT. |
| | ALL BARS | | | | |
| #3 | 17" | 17" | 22" | 17" | 22" |
| #4 | 22" | 22" | 30" | 22" | 30" |
| #5 | 29" | 29" | 36" | 29" | 36" |

CONSTRUCTIV

| Revisions: | | | |
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| # DATE | DE | SCRIPTION OF CHA | NGE |
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| ALL RIGHTS RESERV UTILIZED IN ANY FOI | , /ED. NO PART OF THI RM WITHOUT PRIOR | S DOCUMENT MAY BE WRITTEN AUTHORIZAT | REPRODUCED OR TON OF "COBALT". |
| | | | |
| THIS DRAW INTENDED TO SHALL IT BE SPECIFICS S | ING IS A CONC O BE USED AS A USED FOR PEF HALL BE PREP | EPTUAL PLAN AI A SITE SPECIFIC RMITTING PURPO ARED ON A CAS | ND IS NOT PLAN NOR DSES. SITE E-BY-CASE |
| | INTENT IS P | ROHIBITED. | ENTIONED |
| | | | |
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| DRAWN BY: | AW | CHECKED BY: | ССН |
| PROJECT #: | | SCALE: | N.T.S. |
| DATE: | | | GN-2.00 |

ALL OPTIONS

ALL OPTIONS

LEGEND

15'-1 3/4" 1> INFILL (2) CELLS AT EACH SIDE OF OPENINGS (TYP.) OPENING, (F.V.) TOP PLATE CONNECTION DETAIL (TYP.) UU/F-2.00 OPENING _____(F.V.)_____ 15'-8"

| R | evisions: | | | |
|---------------------------|--|--|---|--|
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| ALL F UTILIZ | RIGHTS RESERV ED IN ANY FOR | ED. NO PART OF TH M WITHOUT PRIOR | IS DOCUMENT MAY B WRITTEN AUTHORIZA | E REPRODUCED OR TION OF "COBALT". |
| T IN SH SF B/ | THIS DRAW TENDED TO IALL IT BE PECIFICS S ASIS AND A | NG IS A CONC) BE USED AS USED FOR PEF HALL BE PREP NY USE BEYO INTENT IS P | EPTUAL PLAN A A SITE SPECIFIC RMITTING PURP ARED ON A CAS ND THE AFOREM ROHIBITED. | ND IS NOT C PLAN NOR OSES. SITE SE-BY-CASE MENTIONED |
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N

RAFTER PLAN (STANDARD) 02 ALL OPTIONS

| | HAN MEMBER SIZE | | | | LEGEND |
|------------------|--------------------|--------------------------------|-----------------|-----------------------|--|
| | 2X6 | LUS26 OR LUC267 CONCEALED | | | |
| $\overline{)}$ | (2) 2X6 | | - | | |
| | 228 | | | | DETL. #4 ON DWG. SD-2.00 |
| | (2) 270 | | - | | PONY WALL TO ROOF DIAPHRAGM |
| (4) (5) | 2) 2/10 | | | | |
| | (2) 2X10 | | | • | BRACE POINT |
| $\overline{(0)}$ | (2) 2X 10 | HUS210-2 OR HUC210-2 CONCEALED | | | BW - BRACE TO WALL |
| (I) | SOME MO | HUS210 OR LUC210Z CONCEALED | | | BB - BRACE TO BEAM |
| | (SEE FRAM | IING PLAN FOR LOCATION) | | | BS - BRACE TO STRONGBACK |
| | | | | | |
| | | | | | |
| | | | | | |
| | | | | | NOTES |
| | | | | CEILING F | RAMING: |
| | | | 1 | . ALL CE | ILING JOISTS SHALL BE 2X6 #2 SYP 16" |
| | | \square | | 0.0.(0 | |
| | | | | 2. ATTIC A 350LB (| ACCESS SHALL BE NO LESS THAN CAPACITY FOLDING STAIR SYSTEM. |
| | BS BS | | 1 | HEADER | FRAMING: |
| | | | | 1. REFER | TO DETAIL 1 ON SD-1.00 FOR ALL |
| | | | | SPECI | FIED ON PLAN) |
| ВW | | | | | |
| | | | | 1. (4) 2X SOLID | STUD PACK UNDER EACH END OF 2X SAWN BEAM. |
| | | | F | | |
| BW | | | 1 | . RIDGES | 5 TO BE 2x10 #2 SYP |
| | | | 2 | 2. ALL HIF | PS TO BE 2x8 #2 SYP |
| PC | | | 3 | 8. ALL RA | FTERS TO BE 2x6 #2 SYP 16"O.C. (UNO) |
| • | | | 4 | . ROOF S | SHEATHING: ¹⁹ 32" APA RATED |
| | | | | PLYWO AROUN | OD W/ 0.120 X 3" RING SHANK NAILS, 4" ID PERIMETER OF DIAPHRAGM. 6" |
| BS | | | | EDGE & | & 6" FIELD NAIL PATTERN. PER |
| • | | | | | |
| | | | 5 | ROOF U R905.1. | 1.1 & TABLE R905.1.1.1 |
| BS | | | F | RAMING: | |
| • | | | 1 | . ALL ST | UDS, FLOOR JOISTS, RAFTERS, TRUSS, |
| | | | | DIRECT | TLY ON EACH OTHER. |
| BS | | | R | evisions: | |
| 6: | 12 | | # | DATE | DESCRIPTION OF CHANGE |
| DC | - | | | | |
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| BS | | | ALL F UTILIZ | RIGHTS RESERN | VED. NO PART OF THIS DOCUMENT MAY BE REPRODUCED OR RM WITHOUT PRIOR WRITTEN AUTHORIZATION OF "COBALT". |
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| | | | _ | | |
| BS | | | | TENDED T | O BE USED AS A SITE SPECIFIC PLAN NOR |
| • | | | SH | HALL IT BE | USED FOR PERMITTING PURPOSES. SITE SHALL BE PREPARED ON A CASE-BY-CASE |
| _BS | | | B | ASIS AND A | ANY USE BEYOND THE AFOREMENTIONED |
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| BS R | | | | | |
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PROJECT #: DATE:

SCALE:

| KING AND JACK STUD SCHEDULE | | | | |
|-----------------------------|---|--|--|--|
| OPENING SPAN | KING/JACK STUDS REQUIRED PER OPENING SPAN | | | |
| 4' OR LESS | (1) KING & (1) JACK STUD (EACH SIDE) | | | |
| 4' TO 6' OPENING | (2) KING & (2) JACK STUD (EACH SIDE) | | | |
| 6' TO 8' OPENING | (3) KING & (2) JACK STUD (EACH SIDE) | | | |
| 8' TO 12' OPENING | (4) KING & (3) JACK STUD (EACH SIDE) | | | |
| 12' TO 16' OPENING | (5) KING & (4) JACK STUD (EACH SIDE) | | | |

SINGLE STORY HEADER SCHEDULE

| ALL H | EADERS ARE DROP BEA | MS, U.N.O | GRADE # 2 LUMBER | |
|-------|---------------------|-----------------|--|--|
| TYPE | OPENING SPAN | WALL THICKNE | HEADER REQ'D SS | |
| 1 | 3' OR LESS | 3.5" | TWO 2"x8" # 2 SYP w/ 1/2" PLYWD FLITCH | |
| 2 | 3' OR LESS | 5.5" | THREE 2"x8" #2 SYP w/ 1/2" PLYWD FLITCHS | |
| 3 | 3' TO 5' | 3.5" | TWO 2"x10" # 2 SYP w/ 1/2" PLYWD FLITCH | |
| 4 | 3' TO 5' | 5.5" | THREE 2"x10" #2SYP w/ 1/2" PLYWD FLITCHS | |
| 5 | 5' TO 8' | 3.5" | TWO 2"x12" #2 SYP w/ 1/2" PLYWD FLITCH | |
| 6 | 5' TO 8' | 5.5" | THREE 2"x12" #2 SYP w/ 1/2" PLYWD FLITCHS | |
| | | | | |

OPENINGS GREATER THAN 6'-1" SHALL BE AS INDICATED ON PLAN DRAWINGS NOTES:

THIS TABLE USES EITHER 0.25" DEFLECTION OR L/240 WHICHEVER IS LESS 1

THIS TABLE ACCOUNTS FOR ONLY ROOF AND CEILING LOADING 2.

3. FOR GENERIC 1 STORY FRAMING 4. FOR 2-STORY DWELLINGS USE 2X12 HEADERS ON FIRST FLOOR LIVING SPACE (UNO)

ALL NON-LOAD BEARING HEADERS SHALL BE 2X6 #2 SYP W/ $\frac{1}{2}$ " PLYWOOD FLITCH. *

INTEREIOR HEADER SCHEDULES 1

(3) BEAM TO FILLED CMU WALL DETAIL

SCHEDULE

CLIP & STRAP SCHEDULE

| CONNECTION LOCATION | CLIP / STRAP |
|--------------------------------|---------------------|
| STUD TO BOTTOM PLATE (SLAB) | SIMPSON H2.5A 1 |
| STUD TO STRINGER/BEAM (PILING) | SIMPSON LSTA36 23 |
| STUD TO STUD BETWEEN LEVELS | SIMPSON LSTA36 23 |
| HEADER STRAPS (8' PLATE) | SIMPSON LSTA36 13 |
| HEADER STRAPS (9' PLATE) | SIMPSON CS16 (48") |
| 2X RAFTER TO TOP PLATE/ BEAM | SIMPSON H2.5A 1 |
| RAFTER TO RAFTER @ RIDGE | SIMPSON LSTA18 1 |
| RAFTER TO RAFTER @ HIP | SIMPSON LSTA18 1 |
| ROOF TRUSS TO STUD | SIMPSON HTS30C 1 |
| RAFTER TO TOP PLATE | (2) SIMPSON H2.5A 1 |
| STUD TO TOP PLATE | (2) SIMPSON H2.5A 1 |
| | |

1. FILL ALL NAIL HOLES U.N.O.

- 2. STUD STRAPS SHALL TERMINATE AT A BEAM OR STRINGER (NEVER AT BAND JOIST) & EXTEND PAST RIM JOIST BY A MINIMUM OF 10".
- 3. SIMPSON CS16 (OF EQUAL LENGTH AND FASTENERS) MAY BE SUBSTITUTED FOR SIMPSON LSTA
- 4. FASTENERS & QUANTITY PER MANUFACTURER'S
- INSTALLATION INSTRUCTIONS.
- 5. SOME CLIPS & STRAPS LISTED MAY NOT APPLY TO EVERY PROJECT.

| R | evisions: | | |
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THIS DRAWING IS A CONCEPTUAL PLAN AND IS NOT INTENDED TO BE USED AS A SITE SPECIFIC PLAN NOR SHALL IT BE USED FOR PERMITTING PURPOSES. SITE SPECIFICS SHALL BE PREPARED ON A CASE-BY-CASE BASIS AND ANY USE BEYOND THE AFOREMENTIONED INTENT IS PROHIBITED.

| DRAWN BY: | AW | CHECKED BY: | ССН |
|------------|----|-------------|---------|
| PROJECT #: | | SCALE: | N.T.S. |
| DATE | | | SD-1.00 |

